LONGITUDE
ONE TWENTY°
ENGINEERING & DESIGN

Structural Package for:

Litchfield Residence

9001 SE 50th St
Mercer Island, WA 98040

Project No: S221118-2

April 3, 2023

STRUCTURAL ENGINEER
L120 ENGINEERING & DESIGN
13150 91ST PL NE
KIRKLAND, WA 98034
CONTACT: MANS THURFJELL, PE
PHONE: 425-636-3313
MTHURFJELL@L120ENGINEERING.COM

Project Number:	Plan Name:	Sheet Number:
S221118-2	Litchfield Residence	DC
Engineer:	Specifics:	Date:
HK	Design Criteria	4/2/2023

Gravity Criteria: BLUE = Review and update as required - Typical Input Code: IBC 2018

ROOF SYSTEM		
Live Load:		
Snow	25.0	psf
Dead Load:		
Composite Roofing	2.0	psf
19/32" Plywood Sheathing	2.5	psf
Trusses at 24" o.c.	3.0	psf
Insulation	1.8	psf
(2) Layers 5/8" GWB	4.4	psf
Misc or Tile Roof	1.3	psf
Total	**15.0**	**psf**

FLOOR SYSTEM		
Live Load:		
Residential	40.0	psf
Dead Load:		
Flooring	3.0	psf
3/4" T & G Plywood	2.5	psf
Floor Joists at 16" o.c.	2.5	psf
Insulation	0.5	psf
(1) Layers 5/8" GWB	2.2	psf
Misc or Tile Flooring	1.3	psf
Total	**12.0**	**psf**

EXTERIOR WALL SYSTEM		
2x6 at 16" o.c.	1.7	psf
Insulation	1.0	psf
1/2" Plywood Sheathing	1.5	psf
(2) layers 5/8" GWB	4.4	psf
Misc or Brick Covered Wall	3.4	psf
Total	**12.0**	**psf**

INTERIOR WALL SYSTEM		
2x4 at 16" o.c.	1.1	psf
Insulation	0.5	psf
(2) Layers 5/8" GWB	4.4	psf
Misc	2.0	psf
Total	**8.0**	**psf**

SEISMIC PARAMETERS:

Code Reference: ASCE 7-16
R = 6.5 Bearing Wall System, Wood Structural Panel Walls
Mapped Spectral Acceleration, Ss = 1.6
Mapped Spectral Acceleration, S1 = 0.63
Soil Site Class = D

WIND PARAMETERS:

Code Reference: ASCE 7-16
Basic Wind Speed (3 second Gust) = 100 mph
Exposure : B
Kzt = 1.60

SOIL PARAMETERS:

Soil Bearing Pressure = 1,500 psf competent native soil or structural fill
1/3 increase for short-term wind or seismic loading is acceptable
Frost Depth = 18 in

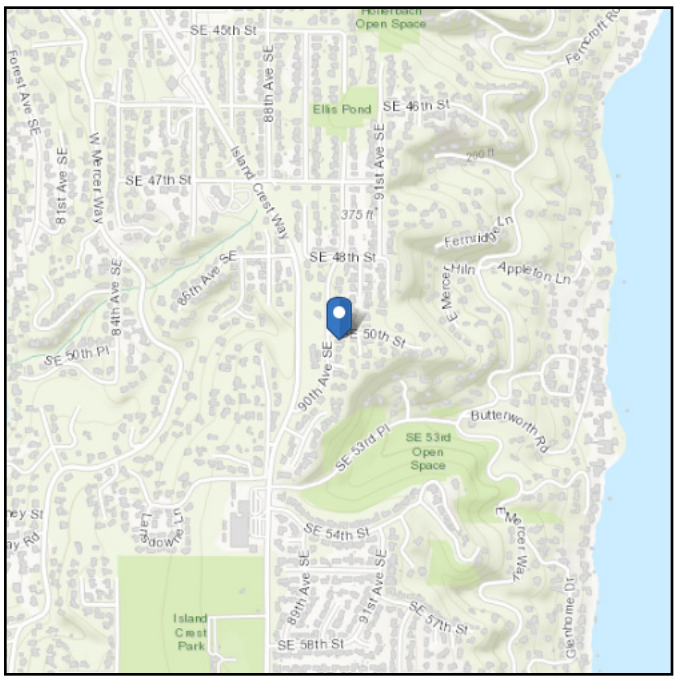
Lateral Wall Pressures:
Unrestrained Active Pressure = 35 pcf Cantilevered walls
Restrained Active Pressure = 50 pcf Plate Wall Design/Tank Walls
Passive Pressure = 250 pcf
Soil Friction Coeff. = 0.35

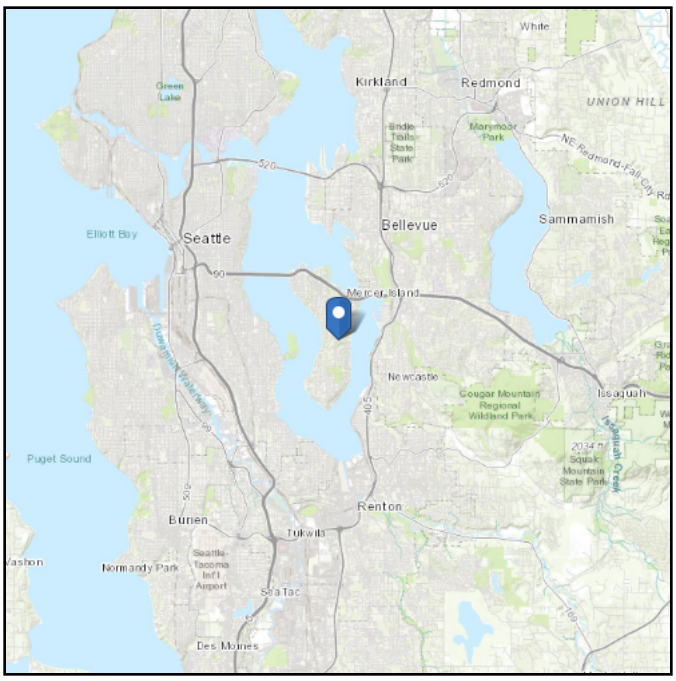
ASCE 7 Hazards Report

Address:
9001 SE 50th St
Mercer Island, Washington
98040

Standard: ASCE/SEI 7-22
Risk Category: II
Soil Class: Default

Latitude: 47.558063
Longitude: -122.219091
Elevation: 357.06 ft (NAVD 88)





Wind

Results:

Wind Speed	98 Vmph
10-year MRI	67 Vmph
25-year MRI	74 Vmph
50-year MRI	78 Vmph
100-year MRI	83 Vmph
300-year MRI	92 Vmph
700-year MRI	98 Vmph
1,700-year MRI	105 Vmph
3,000-year MRI	109 Vmph
10,000-year MRI	118 Vmph
100,000-year MRI	136 Vmph
1,000,000-year MRI	154 Vmph

Data Source: ASCE/SEI 7-22, Fig. 26.5-1B and Figs. CC.2-1–CC.2-4, and Section 26.5.2
Date Accessed: Fri Jan 27 2023

Seismic

Default

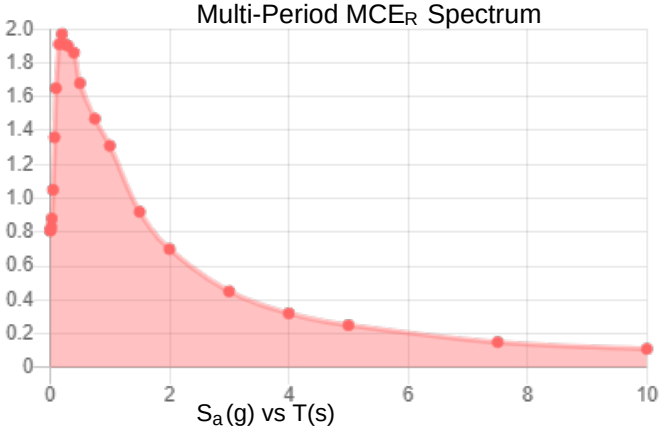
Site Soil Class:

Results:

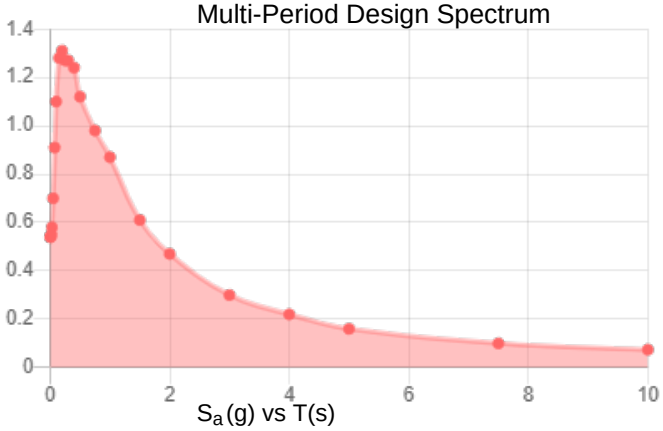
PGA_M :	0.74	T_L :	6
S_{MS} :	1.77	S_S :	1.6
S_{M1} :	1.31	S_1 :	0.63
S_{DS} :	1.18	V_{S30} :	260
S_{D1} :	0.87		

Seismic Design Category: D

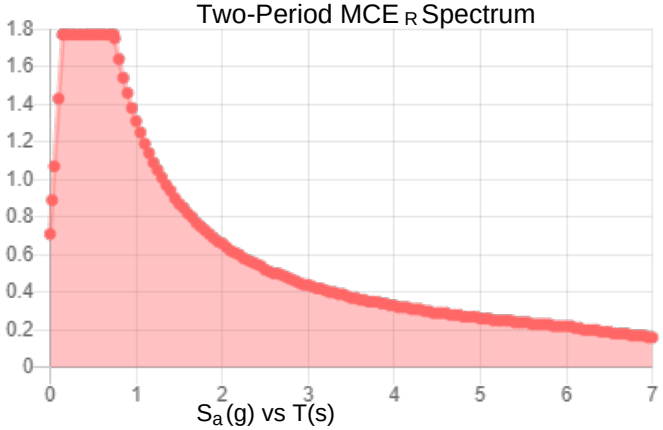
Multi-Period MCE_R Spectrum



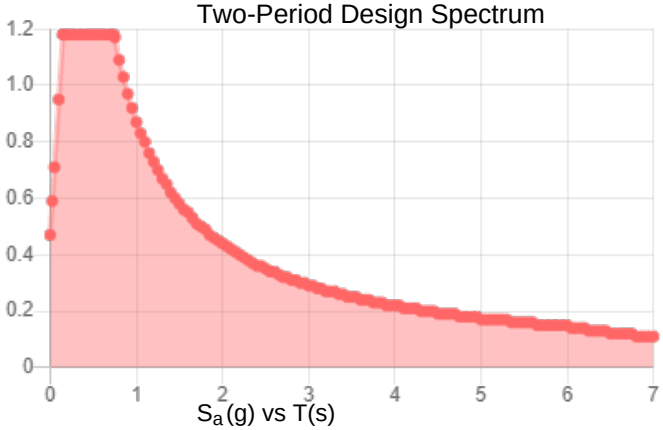
Multi-Period Design Spectrum



Two-Period MCE_R Spectrum



Two-Period Design Spectrum



MCE_R Vertical Response Spectrum

Vertical ground motion data has not yet been made available by USGS.

Design Vertical Response Spectrum

Vertical ground motion data has not yet been made available by USGS.

LONGITUDE
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FRAMING CALCULATIONS

BEAM REFERENCE PER PLAN

(425) 636 3313
L120Engineering.com

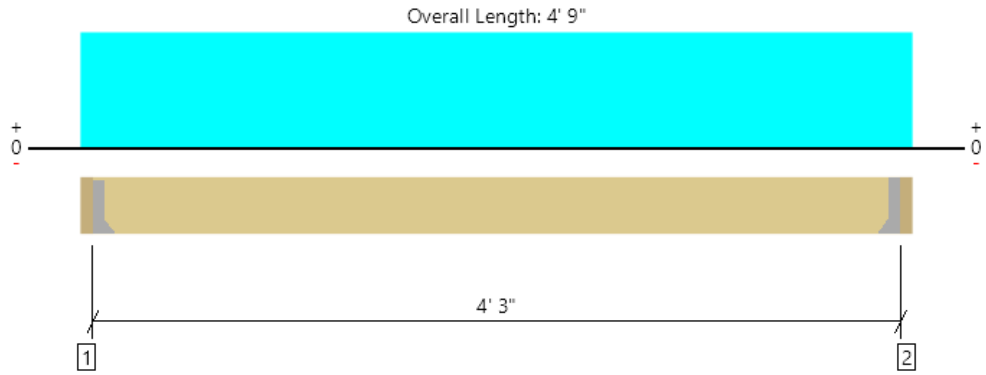
Roof			
Member Name	Results	Current Solution	Comments
RB-1 (skylight header)	Passed	2 piece(s) 2 x 8 DF No.2	
VT-1 (For Reactions Only)	Passed	2 piece(s) 1 3/4" x 14" 2.0E Microllam® LVL	
GT-1 (For Reactions Only)	Passed	1 piece(s) 3 1/2" x 11 7/8" 2.2E Parallam® PSL	
GT-2 (For Reactions Only)	Passed	1 piece(s) 3 1/2" x 16" 2.2E Parallam® PSL	
RH-1	Passed	1 piece(s) 4 x 8 DF No.2	
RH-2	Passed	2 piece(s) 2 x 8 HF No.2	
RH-3	Passed	2 piece(s) 2 x 8 HF No.2	
RH-4	Passed	2 piece(s) 2 x 8 HF No.2	
2nd Floor			
Member Name	Results	Current Solution	Comments
2J-1	Passed	2 piece(s) 9 1/2" TJI® 230 @ 16" OC	
2J-2	Passed	1 piece(s) 9 1/2" TJI® 210 @ 12" OC	
2B-1	Passed	1 piece(s) 5 1/4" x 18" 2.2E Parallam® PSL	
2B-1 (Steel Opt)	Passed	1 piece(s) W10X33 (A992) ASTM Steel	
2B-2	Passed	1 piece(s) 5 1/4" x 9 1/2" 2.2E Parallam® PSL	
2B-3	Passed	1 piece(s) 5 1/4" x 9 1/2" 2.2E Parallam® PSL	
2B-4	Passed	1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL	
2B-5	Passed	1 piece(s) 5 1/4" x 18" 2.2E Parallam® PSL	
2B-5 (Steel Opt)	Passed	1 piece(s) W10X33 (A992) ASTM Steel	
2B-6	Passed	1 piece(s) 7" x 20" 2.2E Parallam® PSL	
2B-6.1 (opt)	Passed	1 piece(s) 5 1/4" x 18" 2.2E Parallam® PSL	
2B-6.2 (opt)	Passed	1 piece(s) 5 1/4" x 14" 2.2E Parallam® PSL	
2B-6 (Steel Opt)	Passed	1 piece(s) W10X45 (A992) ASTM Steel	
2B-7	Passed	1 piece(s) 5 1/4" x 9 1/2" 2.2E Parallam® PSL	
2B-8	Passed	1 piece(s) 7" x 20" 2.2E Parallam® PSL	
2B-8 (Steel Opt)	Passed	1 piece(s) W10X45 (A992) ASTM Steel	
2B-9	Passed	1 piece(s) 3 1/2" x 9 1/2" 2.2E Parallam® PSL	
2B-10	Passed	1 piece(s) 5 1/4" x 9 1/2" 2.2E Parallam® PSL	
2B-11	Passed	1 piece(s) 5 1/4" x 20" 2.2E Parallam® PSL	
2B-11 (Steel Opt)	Passed	1 piece(s) W12X40 (A992) ASTM Steel	
1st Floor			
Member Name	Results	Current Solution	Comments
1H-1 (Garage Header)	Passed	1 piece(s) 3 1/2" x 9 1/2" 2.2E Parallam® PSL	
1H-2	Passed	1 piece(s) 3 1/2" x 9" 24F-V4 DF Glulam	
1J-1 (Under Exercise Room)	Passed	1 piece(s) 9 1/2" TJI® 210 @ 16" OC	
1J-2	Passed	1 piece(s) 9 1/2" TJI® 210 @ 16" OC	
1B-1	Passed	1 piece(s) 4 x 8 DF No.2	

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

3/1/2023 4:56:49 PM UTC
ForteWEB v3.5
File Name: Litchfield Residence

Roof, RB-1 (skylight header)
2 piece(s) 2 x 8 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	814 @ 3"	2813 (1.50")	Passed (29%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	583 @ 10 1/4"	3002	Passed (19%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	865 @ 2' 4 1/2"	2720	Passed (32%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.011 @ 2' 4 1/2"	0.213	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.018 @ 2' 4 1/2"	0.283	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Roof
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD
Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Hanger on 7 1/4" HF beam	3.00"	Hanger[1]	1.50"	359	549	908	See note [1]
2 - Hanger on 7 1/4" HF beam	3.00"	Hanger[1]	1.50"	359	549	908	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 3" o/c	
Bottom Edge (Lu)	4' 3" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	LUS26-2	2.00"	N/A	4-10d	4-10d	
2 - Face Mount Hanger	LUS26-2	2.00"	N/A	4-10d	4-10d	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	3" to 4' 6"	N/A	5.5	--	
1 - Uniform (PSF)	0 to 4' 9" (Front)	9' 3"	15.8	25.0	Roof Load

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

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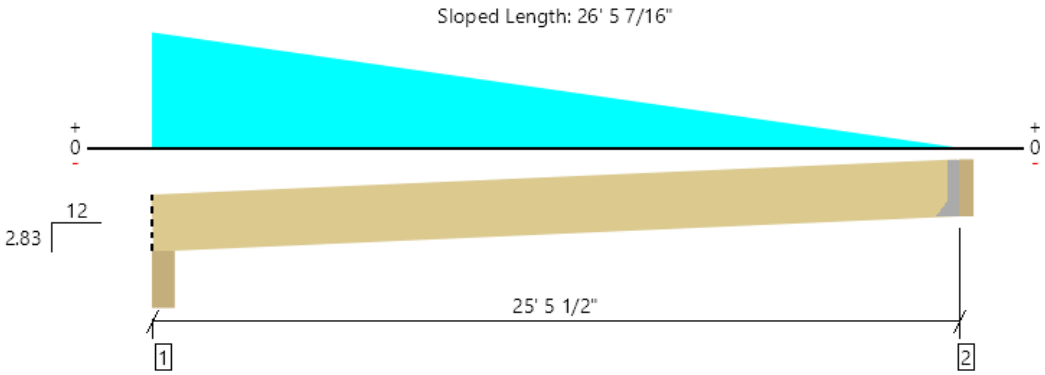
Roof, VT-1 (For Reactions Only)

2 piece(s) 1 3/4" x 14" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Member Length : 26' 5 1/8"

System : Roof
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD
Member Pitch : 2.83/12

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4654 @ 4"	7796 (5.50")	Passed (60%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	3829 @ 1' 7 1/8"	10707	Passed (36%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	21888 @ 11' 1/2"	27897	Passed (78%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.959 @ 12' 5"	1.291	Passed (L/323)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	1.655 @ 12' 5 1/4"	1.721	Passed (L/187)	--	1.0 D + 1.0 S (All Spans)

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Beveled Plate - HF	5.50"	5.50"	3.28"	1919	2735	4654	Blocking
2 - Hanger on 14" DF beam	3.50"	Hanger[1]	1.50"	1016	1315	2331	See note [1]

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	6' 2" o/c	
Bottom Edge (Lu)	26' 2" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	LSSR410Z	1.88"	N/A	22-16dx2.5	18-16dx2.5	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 25' 5 1/2"	N/A	14.3	--	
1 - Tapered (PLF)	0 to 25' 5 1/2"	N/A	195.9 to 0.0	318.2 to 0.0	Generated from Roof Geometry

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

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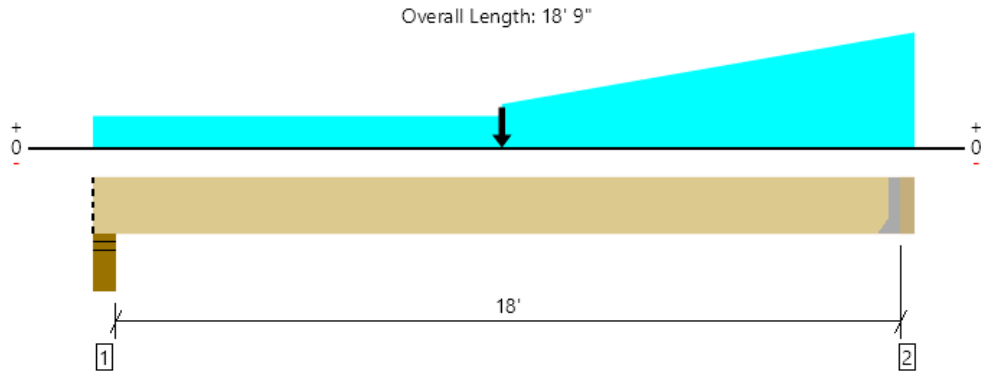
Roof, GT-1 (For Reactions Only)
1 piece(s) 3 1/2" x 11 7/8" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2876 @ 18' 5 1/2"	3281 (1.50")	Passed (88%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	2591 @ 17' 5 5/8"	9241	Passed (28%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	16167 @ 9' 6"	22888	Passed (71%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.463 @ 9' 6"	0.906	Passed (L/470)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.819 @ 9' 6"	1.208	Passed (L/266)	--	1.0 D + 1.0 S (All Spans)

System : Roof
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD
Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Stud wall - HF	5.50"	5.50"	1.57"	979	1242	2221	Blocking
2 - Hanger on 11 7/8" DF beam	3.50"	Hanger[1]	1.50"	1255	1704	2958	See note [1]

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	18' 6" o/c	
Bottom Edge (Lu)	18' 6" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	HHUS48	3.00"	N/A	22-10d	8-10d	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 18' 5 1/2"	N/A	13.0	--	
1 - Uniform (PSF)	0 to 9' 6" (Front)	2'	15.0	25.0	Roof Load
2 - Tapered (PSF)	9' 6" to 18' 9" (Back)	0 to 4' 6"	15.0	25.0	Roof Load
3 - Uniform (PSF)	9' 6" to 18' 9" (Front)	2' 9"	15.0	25.0	Roof Load
4 - Point (lb)	9' 6" (Back)	N/A	1016	1315	Linked from: VT-1 (For Reactions Only), Support 2

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

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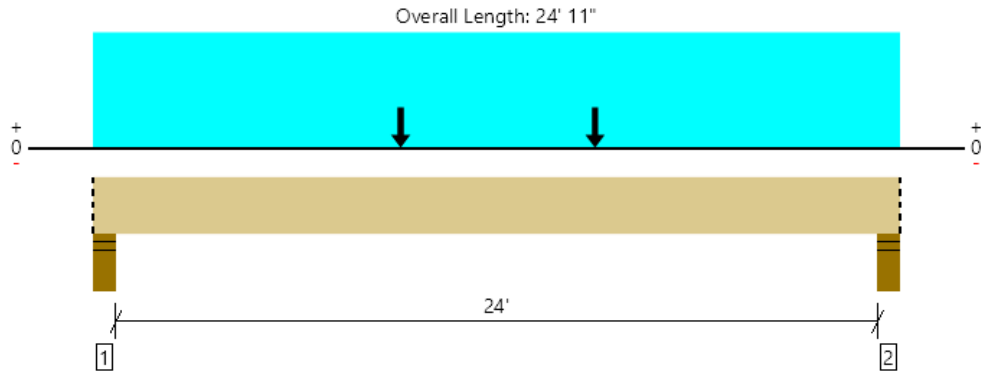
Roof, GT-2 (For Reactions Only)
1 piece(s) 3 1/2" x 16" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4184 @ 24' 7"	7796 (5.50")	Passed (54%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	4009 @ 23' 1 1/2"	12451	Passed (32%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	34168 @ 12' 4 1/4"	40198	Passed (85%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.794 @ 12' 5 9/16"	1.212	Passed (L/367)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	1.411 @ 12' 5 9/16"	1.617	Passed (L/206)	--	1.0 D + 1.0 S (All Spans)

System : Roof
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD
Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Stud wall - HF	5.50"	5.50"	2.94"	1842	2321	4164	Blocking
2 - Stud wall - HF	5.50"	5.50"	2.95"	1851	2333	4184	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	12' o/c	
Bottom Edge (Lu)	24' 11" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 24' 11"	N/A	17.5	--	
1 - Uniform (PSF)	0 to 24' 11" (Front)	2'	15.0	25.0	Roof Load
2 - Point (lb)	9' 6" (Front)	N/A	1255	1704	Linked from: GT-1 (For Reactions Only), Support 2
3 - Point (lb)	15' 6" (Front)	N/A	1255	1704	Linked from: GT-1 (For Reactions Only), Support 2

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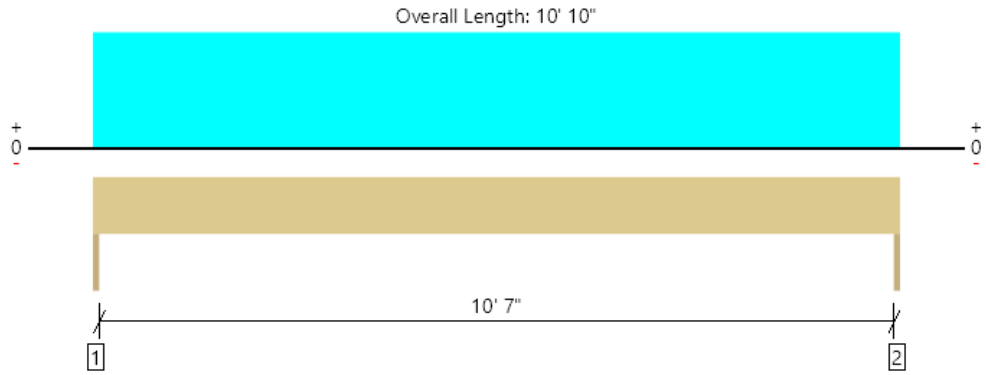
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

Roof, RH-1
1 piece(s) 4 x 8 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	468 @ 0	3281 (1.50")	Passed (14%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	405 @ 8 3/4"	3502	Passed (12%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	1268 @ 5' 5"	3376	Passed (38%)	1.15	1.0 D + 1.0 S (All Spans)
Vert Live Load Defl. (in)	0.087 @ 5' 5"	0.361	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Vert Total Load Defl. (in)	0.151 @ 5' 5"	0.313	Passed (L/863)	--	1.0 D + 1.0 S (All Spans)
Lat Member Reaction (lbs)	158 @ 10' 10"	N/A	Passed (N/A)	1.60	1.0 D + 0.6 W
Lat Shear (lbs)	146 @ 5"	4872	Passed (3%)	1.60	1.0 D + 0.6 W
Lat Moment (Ft-lbs)	429 @ mid-span	2425	Passed (18%)	1.60	1.0 D + 0.6 W
Lat Deflection (in)	0.153 @ mid-span	1.083	Passed (L/850)	--	1.0 D + 0.6 W
Bi-Axial Bending	0.37	1.00	Passed (37%)	1.60	1.0 D + 0.45 W + 0.75 L + 0.75 S

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (5/16").
- Lateral deflection criteria: Wind (L/120)
- A 1.8% decrease in the moment capacity has been added to account for lateral stability.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - HF	1.50"	1.50"	1.50"	197	271	468	None
2 - Trimmer - HF	1.50"	1.50"	1.50"	197	271	468	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Lateral Connections						
Supports	Plate Size	Plate Material	Connector	Type/Model	Quantity	Nailing
Left	2X	Hem Fir	Nails	8d (0.113" x 2 1/2") (Toe)	2	
Right	2X	Hem Fir	Nails	8d (0.113" x 2 1/2") (Toe)	2	

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 10' 10"	N/A	6.4	--	
1 - Uniform (PSF)	0 to 10' 10"	2'	15.0	25.0	Roof Load

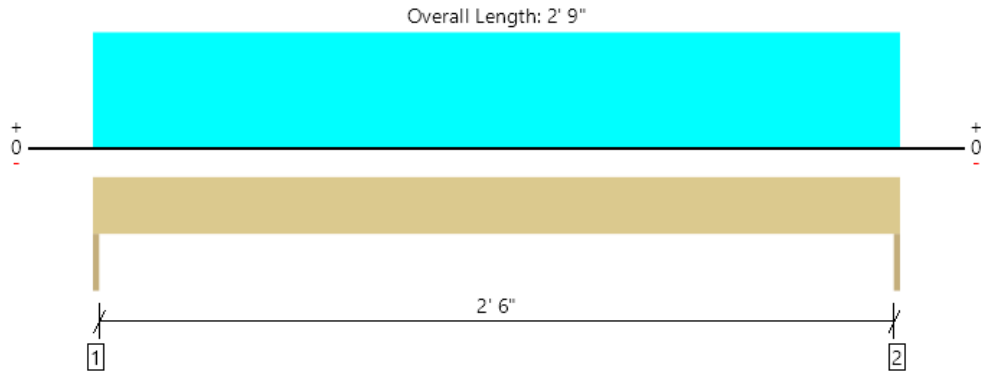
Lateral Load	Location	Tributary Width	Wind (1.60)	Comments
1 - Uniform (PSF)	Full Length	2'	24.4	

- ASCE/SEI 7 Sec. 30.4: Exposure Category (B), Mean Roof Height (33'), Topographic Factor (1.0), Wind Directionality Factor (0.85), Basic Wind Speed (115), Risk Category(II), Effective Wind Area determined using full member span and trib. width.
- IBC Table 1604.3, footnote f: Deflection checks are performed using 42% of this lateral wind load.

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

Roof, RH-2
2 piece(s) 2 x 8 HF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	457 @ 0	1823 (1.50")	Passed (25%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	214 @ 8 3/4"	2501	Passed (9%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	314 @ 1' 4 1/2"	2520	Passed (12%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.002 @ 1' 4 1/2"	0.092	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.003 @ 1' 4 1/2"	0.138	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- A 1.9% decrease in the moment capacity has been added to account for lateral stability.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - HF	1.50"	1.50"	1.50"	182	275	457	None
2 - Trimmer - HF	1.50"	1.50"	1.50"	182	275	457	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 2' 9"	N/A	5.5	--	
1 - Uniform (PSF)	0 to 2' 9"	8'	15.8	25.0	Roof Load

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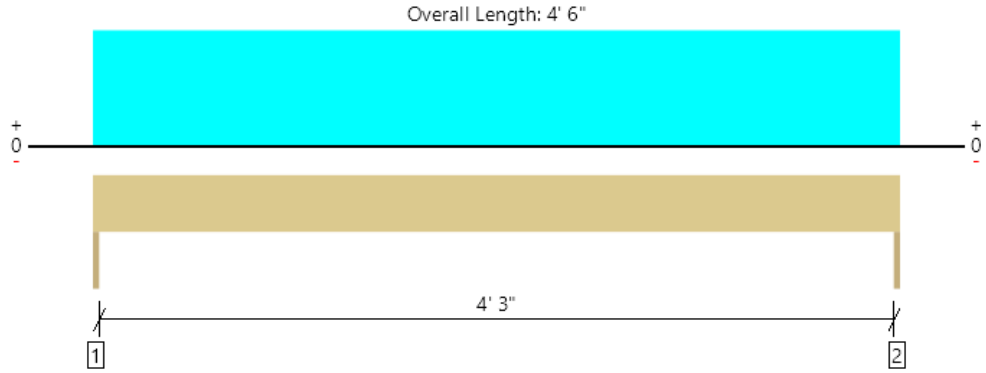
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

Roof, RH-3
2 piece(s) 2 x 8 HF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1182 @ 0	1823 (1.50")	Passed (65%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	799 @ 8 3/4"	2501	Passed (32%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	1330 @ 2' 3"	2470	Passed (54%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.024 @ 2' 3"	0.150	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.039 @ 2' 3"	0.225	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- A 3.9% decrease in the moment capacity has been added to account for lateral stability.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - HF	1.50"	1.50"	1.50"	451	731	1182	None
2 - Trimmer - HF	1.50"	1.50"	1.50"	451	731	1182	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 4' 6"	N/A	5.5	--	
1 - Uniform (PSF)	0 to 4' 6"	13'	15.0	25.0	Roof Load

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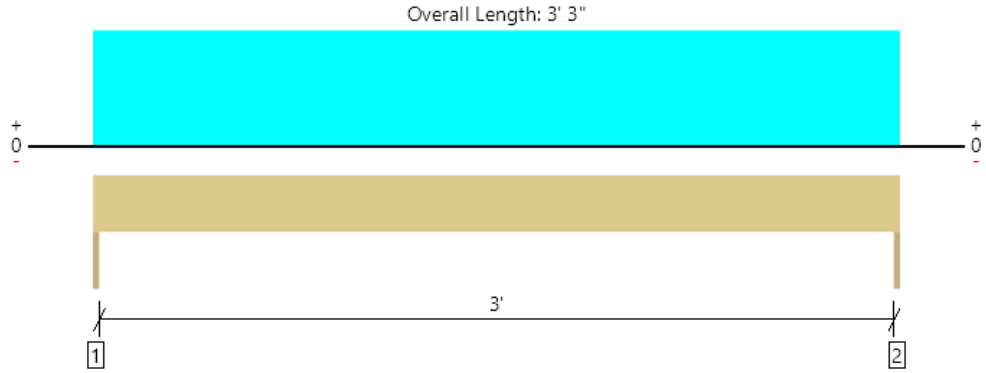
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

Roof, RH-4

2 piece(s) 2 x 8 HF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	606 @ 0	1823 (1.50")	Passed (33%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	334 @ 8 3/4"	2501	Passed (13%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	492 @ 1' 7 1/2"	2507	Passed (20%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.005 @ 1' 7 1/2"	0.108	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.008 @ 1' 7 1/2"	0.162	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- A 2.4% decrease in the moment capacity has been added to account for lateral stability.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - HF	1.50"	1.50"	1.50"	240	366	606	None
2 - Trimmer - HF	1.50"	1.50"	1.50"	240	366	606	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 3' 3"	N/A	5.5	--	
1 - Uniform (PSF)	0 to 3' 3"	9'	15.8	25.0	Roof Load

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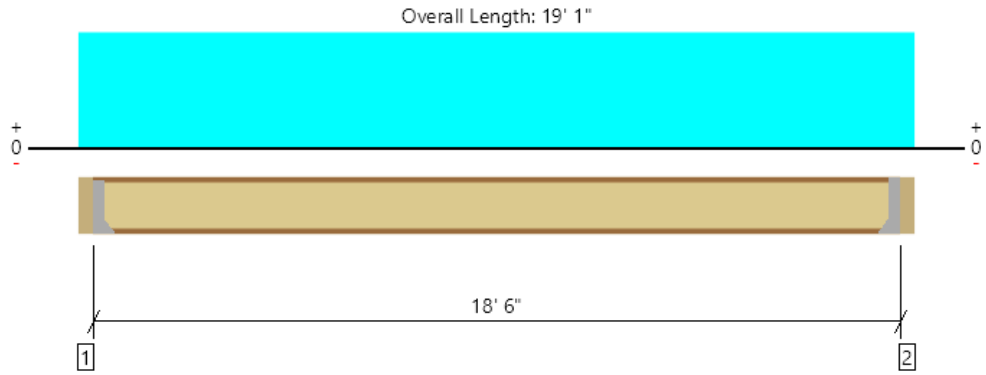
2nd Floor, 2J-1
2 piece(s) 9 1/2" TJI® 230 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	641 @ 3 1/2"	2120 (1.75")	Passed (30%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	641 @ 3 1/2"	2660	Passed (24%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2966 @ 9' 6 1/2"	6660	Passed (45%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.325 @ 9' 6 1/2"	0.463	Passed (L/683)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.423 @ 9' 6 1/2"	0.925	Passed (L/525)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	41	40	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: None.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Hanger on 9 1/2" DF beam	3.50"	Hanger[1]	1.75" / - [2]	153	509	662	See note [1]
2 - Hanger on 9 1/2" DF beam	3.50"	Hanger[1]	1.75" / - [2]	153	509	662	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.
- [2] Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	6' 2" o/c	
Bottom Edge (Lu)	18' 6" o/c	

- TJI joists are only analyzed using Maximum Allowable bracing solutions.
- Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	MIU4.75/9	2.50"	N/A	16-10dx1.5	2-10dx1.5	
2 - Face Mount Hanger	MIU4.75/9	2.50"	N/A	16-10dx1.5	2-10dx1.5	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 19' 1"	16"	12.0	40.0	Floor Load

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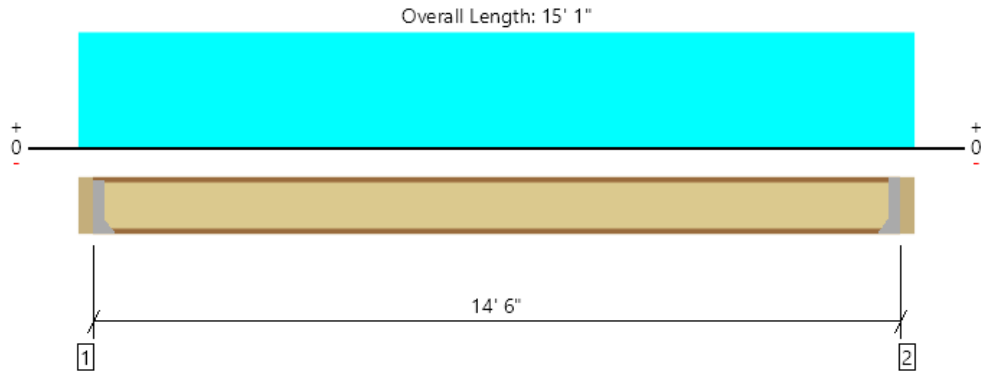
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2J-2
1 piece(s) 9 1/2" TJI® 210 @ 12" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	377 @ 3 1/2"	1005 (1.75")	Passed (38%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	377 @ 3 1/2"	1330	Passed (28%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1367 @ 7' 6 1/2"	3000	Passed (46%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.201 @ 7' 6 1/2"	0.363	Passed (L/866)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.261 @ 7' 6 1/2"	0.725	Passed (L/666)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	42	40	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: None.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Hanger on 9 1/2" DF beam	3.50"	Hanger[1]	1.75" / - [2]	91	302	392	See note [1]
2 - Hanger on 9 1/2" DF beam	3.50"	Hanger[1]	1.75" / - [2]	91	302	392	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.
- [2] Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	5' 7" o/c	
Bottom Edge (Lu)	14' 6" o/c	

- TJI joists are only analyzed using Maximum Allowable bracing solutions.
- Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	IUS2.06/9.5	2.00"	N/A	8-10dx1.5	2-Strong-Grip	
2 - Face Mount Hanger	IUS2.06/9.5	2.00"	N/A	8-10dx1.5	2-Strong-Grip	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 15' 1"	12"	12.0	40.0	Floor Load

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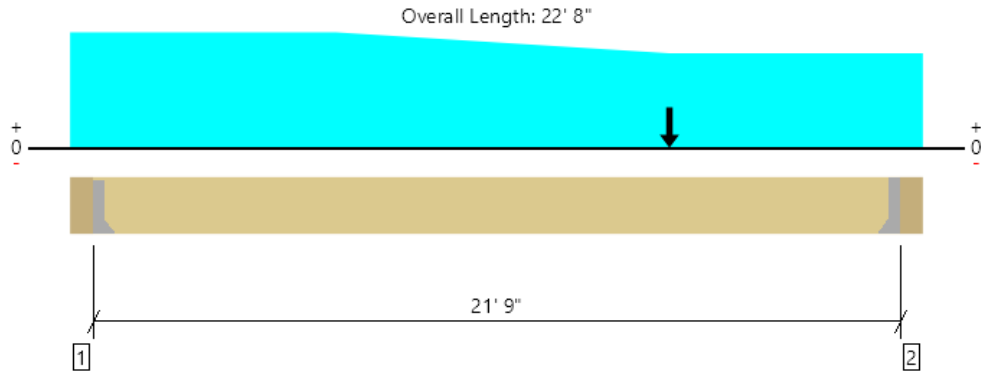
ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-1

1 piece(s) 5 1/4" x 18" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	9625 @ 22' 2 1/2"	9625 (2.93")	Passed (100%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	7562 @ 20' 8 1/2"	18270	Passed (41%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	47168 @ 11' 6 1/2"	65497	Passed (72%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.467 @ 11' 4 11/16"	0.544	Passed (L/559)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.870 @ 11' 4 13/16"	1.087	Passed (L/300)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Hanger on 18" PSL beam	5.50"	Hanger[1]	2.93"	4589	4327	2874	9989	See note [1]
2 - Hanger on 18" PSL beam	5.50"	Hanger[1]	2.93"	4612	4327	2779	9940	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	21' 9" o/c	
Bottom Edge (Lu)	21' 9" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	HGUS5.50/14	4.00"	N/A	66-10d	22-10d	
2 - Face Mount Hanger	HGUS5.50/14	4.00"	N/A	66-10d	22-10d	

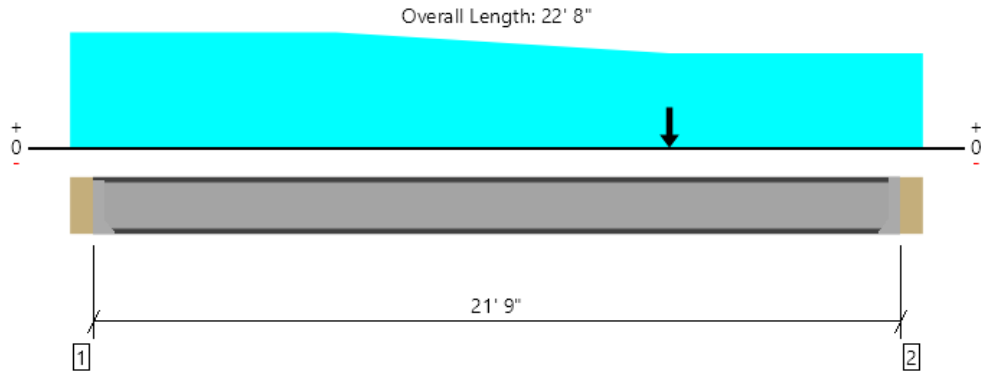
- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	5 1/2" to 22' 2 1/2"	N/A	29.5	--	--	
1 - Uniform (PSF)	0 to 22' 8" (Back)	1'	12.4	-	25.0	Low Roof Load
2 - Uniform (PLF)	0 to 22' 8" (Top)	N/A	100.0	-	-	Wall Load Above
3 - Uniform (PSF)	0 to 7' (Top)	9'	15.8	-	25.0	Roof Load
4 - Tapered (PSF)	7' to 16' (Top)	9' to 4' 6"	15.8	-	25.0	Roof Load
5 - Uniform (PSF)	16' to 22' 8" (Top)	4' 6"	15.8	-	25.0	Roof Load
6 - Point (lb)	16' (Top)	N/A	979	-	1242	Linked from: GT-1 (For Reactions Only), Support 1
7 - Uniform (PLF)	0 to 22' 8" (Front)	N/A	114.8	381.8	-	Linked from: 2J-1, Support 1

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-1 (Steel Opt)
1 piece(s) W10X33 (A992) ASTM Steel



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	10027 @ 5 1/2"	31051 (1.50")	Passed (32%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	9663 @ 22' 2 1/2"	56434	Passed (17%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	53305 @ 11' 8 1/8"	61189	Passed (87%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.492 @ 11' 4 11/16"	0.544	Passed (L/530)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.922 @ 11' 4 13/16"	1.087	Passed (L/283)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Bearing reinforcement may be required for support located at 0".
- Bearing reinforcement may be required for support located at 21' 9".
- Applicable calculations are based on ANSI/AISC 360-16.
- A lateral-torsional buckling factor (Cb) of 1.0 has been assumed.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Hanger on 9 3/4" PSL beam	5.50"	Hanger[1]	1.50" / - [2]	4627	4327	2874	10027	See note [1]
2 - Hanger on 9 3/4" PSL beam	5.50"	Hanger[1]	1.50" / - [2]	4649	4327	2779	9978	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.
- [2] Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	Connector not found	N/A	N/A	N/A	N/A	
2 - Face Mount Hanger	Connector not found	N/A	N/A	N/A	N/A	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

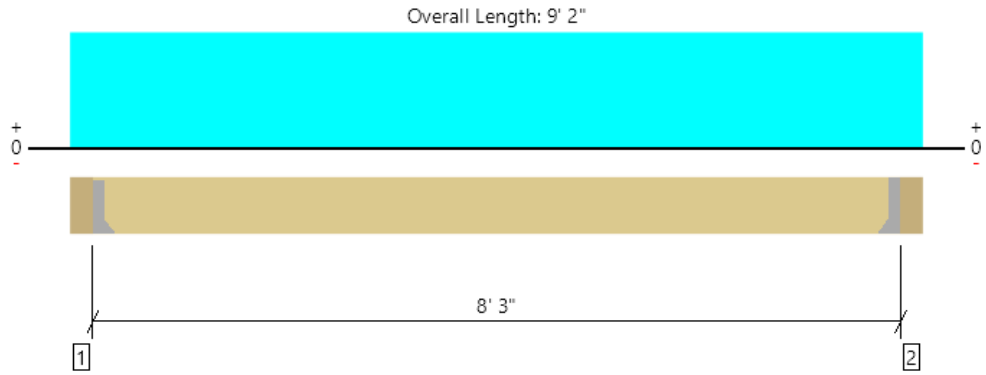
Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	5 1/2" to 22' 2 1/2"	N/A	33.0	--	--	
1 - Uniform (PSF)	0 to 22' 8"	1'	12.4	-	25.0	Low Roof Load
2 - Uniform (PLF)	0 to 22' 8"	N/A	100.0	-	-	Wall Load Above
3 - Uniform (PSF)	0 to 7'	9'	15.8	-	25.0	Roof Load
4 - Tapered (PSF)	7' to 16'	9' to 4' 6"	15.8	-	25.0	Roof Load
5 - Uniform (PSF)	16' to 22' 8"	4' 6"	15.8	-	25.0	Roof Load
6 - Point (lb)	16'	N/A	979	-	1242	Linked from: GT-1 (For Reactions Only), Support 1
7 - Uniform (PLF)	0 to 22' 8"	N/A	114.8	381.8	-	Linked from: 2J-1, Support 1

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-2

1 piece(s) 5 1/4" x 9 1/2" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3103 @ 5 1/2"	4922 (1.50")	Passed (63%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	2508 @ 1' 3"	11089	Passed (23%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	6401 @ 4' 7"	22523	Passed (28%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.054 @ 4' 7"	0.206	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.108 @ 4' 7"	0.412	Passed (L/913)	--	1.0 D + 1.0 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Hanger on 9 1/2" PSL beam	5.50"	Hanger[1]	1.50"	1722	550	1719	3441	See note [1]
2 - Hanger on 9 1/2" PSL beam	5.50"	Hanger[1]	1.50"	1722	550	1719	3441	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	8' 3" o/c	
Bottom Edge (Lu)	8' 3" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	HU612	2.50"	N/A	22-10d	8-10d	
2 - Face Mount Hanger	HU612	2.50"	N/A	22-10d	8-10d	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	5 1/2" to 8' 8 1/2"	N/A	15.6	--	--	
1 - Uniform (PSF)	0 to 9' 2" (Front)	3'	12.0	40.0	-	Floor Load
2 - Uniform (PSF)	0 to 9' 2" (Back)	3'	12.0	-	25.0	Low Roof Load
3 - Uniform (PLF)	0 to 9' 2" (Top)	N/A	100.0	-	-	Wall Load Above
4 - Uniform (PSF)	0 to 9' 2" (Front)	12'	15.8	-	25.0	Roof Load

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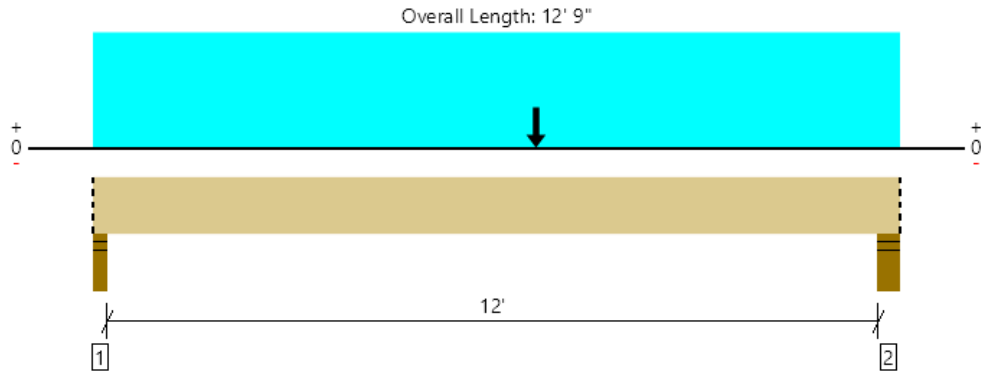
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-3

1 piece(s) 5 1/4" x 9 1/2" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3478 @ 2"	7442 (3.50")	Passed (47%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	4193 @ 11' 6"	11089	Passed (38%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	22015 @ 7'	22523	Passed (98%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.314 @ 6' 5 3/8"	0.408	Passed (L/468)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.626 @ 6' 5 3/8"	0.613	Passed (L/235)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Stud wall - HF	3.50"	3.50"	1.64"	1722	822	1520	3478	Blocking
2 - Stud wall - HF	5.50"	5.50"	2.01"	2125	958	1918	4282	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	12' 9" o/c	
Bottom Edge (Lu)	12' 9" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 12' 9"	N/A	15.6	--	--	
1 - Uniform (PSF)	0 to 12' 9" (Front)	1' 4"	12.0	40.0	-	Floor Load
2 - Point (lb)	7' (Front)	N/A	1722	550	1719	Linked from: 2B-2, Support 2
3 - Point (lb)	7' (Back)	N/A	1722	550	1719	Linked from: 2B-2, Support 1

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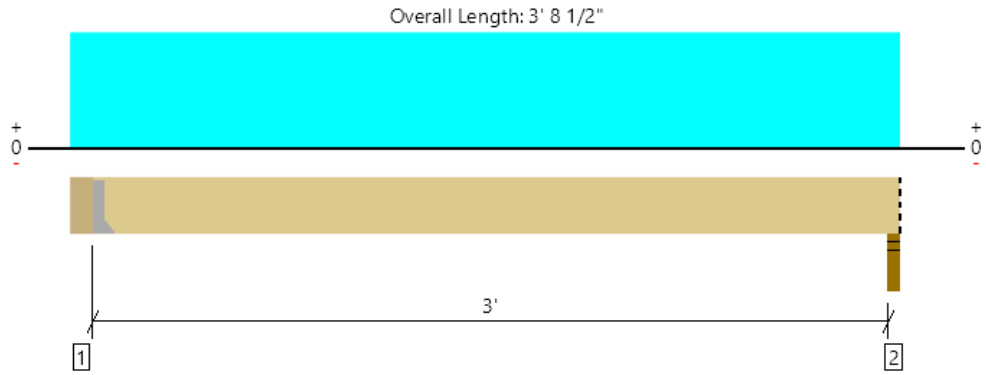
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

2nd Floor, 2B-4

1 piece(s) 1 3/4" x 9 1/2" 2.0E Microllam® LVL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	666 @ 3' 7"	2126 (3.00")	Passed (31%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	304 @ 1' 3"	3159	Passed (10%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	482 @ 2' 1/4"	5887	Passed (8%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.005 @ 2' 1/4"	0.078	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.007 @ 2' 1/4"	0.156	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Hanger on 9 1/2" PSL beam	5.50"	Hanger[1]	1.50"	189	606	796	See note [1]
2 - Stud wall - HF	3.00"	3.00"	1.50"	160	506	666	Blocking

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 3" o/c	
Bottom Edge (Lu)	3' 3" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	IUS1.81/9.5	2.00"	N/A	8-10dx1.5	2-10dx1.5	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
0 - Self Weight (PLF)	5 1/2" to 3' 8 1/2"	N/A	4.8	--	
1 - Uniform (PSF)	0 to 3' 8 1/2" (Front)	7' 6"	12.0	40.0	Floor Load

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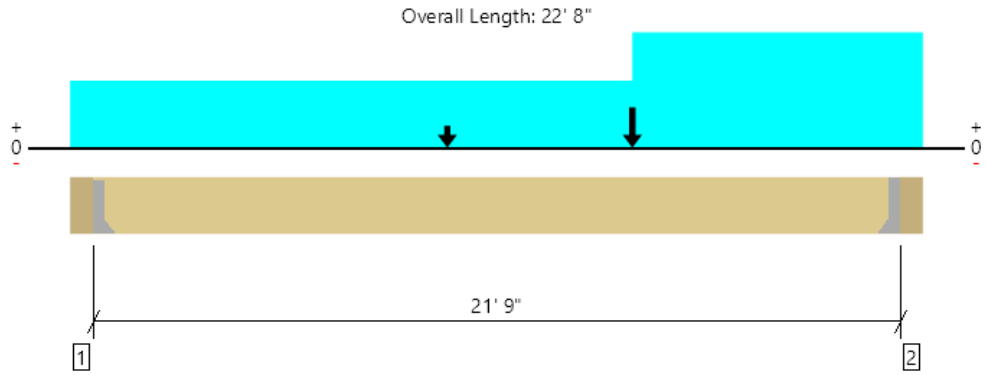
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-5

1 piece(s) 5 1/4" x 18" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	10504 @ 22' 2 1/2"	10504 (3.20")	Passed (100%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	8507 @ 20' 8 1/2"	18270	Passed (47%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	51629 @ 12' 3 3/8"	65497	Passed (79%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.502 @ 11' 7 1/8"	0.544	Passed (L/520)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.838 @ 11' 8 1/4"	1.087	Passed (L/311)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Hanger on 18" PSL beam	5.50"	Hanger[1]	2.51"	2860	5660	898	8520	See note [1]
2 - Hanger on 18" PSL beam	5.50"	Hanger[1]	3.20"	4696	5349	2929	10905	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	21' 9" o/c	
Bottom Edge (Lu)	21' 9" o/c	

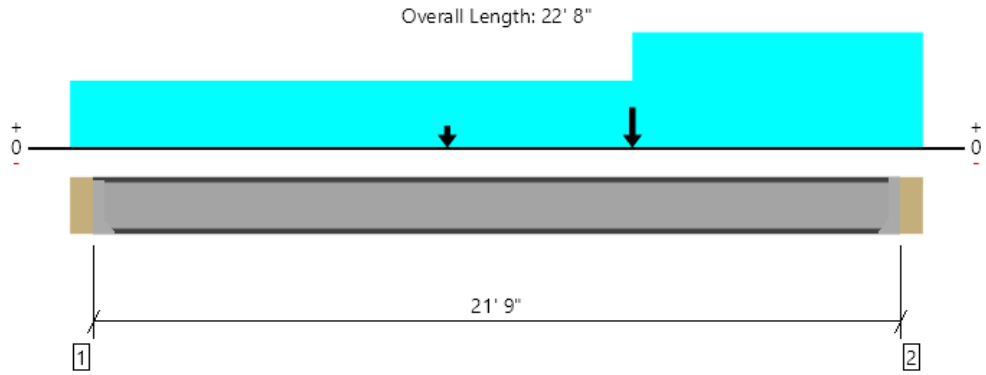
•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	HGUS5.50/14	4.00"	N/A	66-10d	22-10d	
2 - Face Mount Hanger	HGUS5.50/14	4.00"	N/A	66-10d	22-10d	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	5 1/2" to 22' 2 1/2"	N/A	29.5	--	--	
1 - Uniform (PSF)	0 to 15' (Back)	2'	12.0	40.0	-	Floor Load
2 - Uniform (PSF)	15' to 22' 8" (Back)	2'	12.0	-	25.0	Low Roof Load
3 - Uniform (PLF)	15' to 22' 8" (Top)	N/A	100.0	-	-	Wall Load Above
4 - Uniform (PSF)	15' to 22' 8" (Top)	9'	15.8	-	25.0	Roof Load
5 - Uniform (PLF)	0 to 22' 8" (Front)	N/A	114.8	381.8	-	Linked from: 2J-1, Support 2
6 - Point (lb)	10' (Back)	N/A	189	606	-	Linked from: 2B-4, Support 1
7 - Point (lb)	15' (Back)	N/A	1722	550	1719	Linked from: 2B-2, Support 1

2nd Floor, 2B-5 (Steel Opt)
1 piece(s) W10X33 (A992) ASTM Steel



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	10942 @ 22' 2 1/2"	31051 (1.50")	Passed (35%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	10542 @ 22' 2 1/2"	56434	Passed (19%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	52764 @ 13' 10 1/2"	61189	Passed (86%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.529 @ 11' 7 1/8"	0.544	Passed (L/493)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.888 @ 11' 8 1/4"	1.087	Passed (L/294)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Bearing reinforcement may be required for support located at 0".
- Bearing reinforcement may be required for support located at 21' 9".
- Applicable calculations are based on ANSI/AISC 360-16.
- A lateral-torsional buckling factor (Cb) of 1.0 has been assumed.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Hanger on 9 3/4" PSL beam	5.50"	Hanger[1]	1.50" / - [2]	2898	5660	898	8558	See note [1]
2 - Hanger on 9 3/4" PSL beam	5.50"	Hanger[1]	1.50" / - [2]	4734	5349	2929	10942	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.
- [2] Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	Connector not found	N/A	N/A	N/A	N/A	
2 - Face Mount Hanger	Connector not found	N/A	N/A	N/A	N/A	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

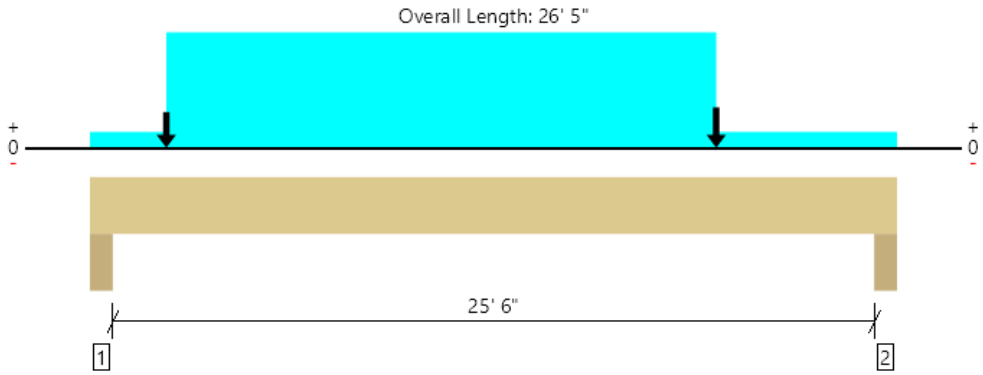
Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	5 1/2" to 22' 2 1/2"	N/A	33.0	--	--	
1 - Uniform (PSF)	0 to 15'	2'	12.0	40.0	-	Floor Load
2 - Uniform (PSF)	15' to 22' 8"	2'	12.0	-	25.0	Low Roof Load
3 - Uniform (PLF)	15' to 22' 8"	N/A	100.0	-	-	Wall Load Above
4 - Uniform (PSF)	15' to 22' 8"	9'	15.8	-	25.0	Roof Load
5 - Uniform (PLF)	0 to 22' 8"	N/A	114.8	381.8	-	Linked from: 2J-1, Support 2
6 - Point (lb)	10'	N/A	189	606	-	Linked from: 2B-4, Support 1
7 - Point (lb)	15'	N/A	1722	550	1719	Linked from: 2B-2, Support 1

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-6

1 piece(s) 7" x 20" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	14650 @ 4"	24063 (5.50")	Passed (61%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	12834 @ 2' 1 1/2"	27067	Passed (47%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	59609 @ 18' 9 3/8"	106561	Passed (56%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.406 @ 13' 9 1/4"	0.644	Passed (L/761)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.846 @ 13' 7 11/16"	1.288	Passed (L/365)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Member should be side-loaded from both sides of the member or braced to prevent rotation.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Trimmer - HF	5.50"	5.50"	3.35"	7422	5530	4107	14650	None
2 - Trimmer - HF	5.50"	5.50"	2.75"	5921	4866	3256	12013	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	26' 5" o/c	
Bottom Edge (Lu)	26' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 26' 5"	N/A	43.8	--	--	
1 - Uniform (PSF)	2' 6" to 20' 6" (Front)	1'	12.0	40.0	-	Floor Load
2 - Uniform (PSF)	0 to 26' 5" (Back)	1'	12.0	-	25.0	Low Roof Load
3 - Uniform (PLF)	2' 6" to 20' 6" (Top)	N/A	100.0	-	-	Wall Load Above
4 - Uniform (PSF)	2' 6" to 20' 6" (Top)	2'	15.8	-	25.0	Roof Load
5 - Point (lb)	2' 6" (Front)	N/A	4589	4327	2874	Linked from: 2B-1, Support 1
6 - Point (lb)	20' 6" (Front)	N/A	4696	5349	2929	Linked from: 2B-5, Support 2

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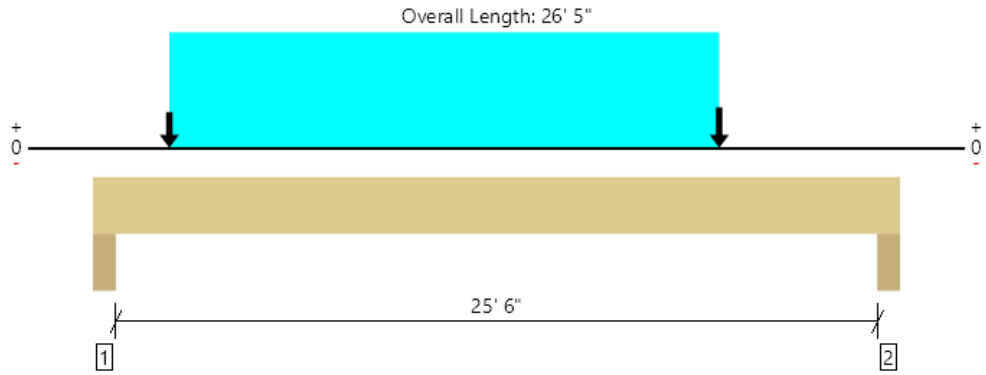
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

2nd Floor, 2B-6.1 (opt)
1 piece(s) 5 1/4" x 18" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	12332 @ 4"	18047 (5.50")	Passed (68%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	11206 @ 1' 11 1/2"	18270	Passed (61%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	52041 @ 20' 6"	65497	Passed (79%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.639 @ 13' 10 9/16"	0.644	Passed (L/483)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	1.174 @ 13' 10"	1.288	Passed (L/263)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Trimmer - HF	5.50"	5.50"	3.76"	5733	5530	3267	12332	None
2 - Trimmer - HF	5.50"	5.50"	3.08"	4548	4866	2536	10099	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	26' 5" o/c	
Bottom Edge (Lu)	26' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 26' 5"	N/A	29.5	--	--	
1 - Uniform (PSF)	2' 6" to 20' 6" (Front)	1'	12.0	40.0	-	Floor Load
2 - Point (lb)	2' 6" (Front)	N/A	4589	4327	2874	Linked from: 2B-1, Support 1
3 - Point (lb)	20' 6" (Front)	N/A	4696	5349	2929	Linked from: 2B-5, Support 2

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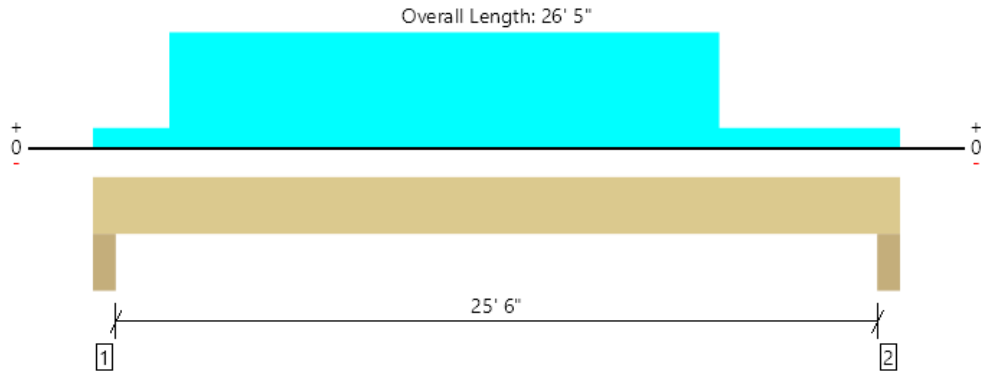
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-6.2 (opt)
1 piece(s) 5 1/4" x 14" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2644 @ 4"	18047 (5.50")	Passed (15%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	2546 @ 1' 7 1/2"	16342	Passed (16%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	18413 @ 12' 9 7/8"	46854	Passed (39%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.266 @ 13' 1 3/16"	0.644	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.845 @ 13' 1"	1.288	Passed (L/366)	--	1.0 D + 1.0 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - HF	5.50"	5.50"	1.50"	1804	840	2644	None
2 - Trimmer - HF	5.50"	5.50"	1.50"	1489	720	2210	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	26' 5" o/c	
Bottom Edge (Lu)	26' 5" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 26' 5"	N/A	23.0	--	
1 - Uniform (PSF)	0 to 26' 5" (Back)	1'	12.0	25.0	Low Roof Load
2 - Uniform (PLF)	2' 6" to 20' 6" (Top)	N/A	100.0	-	Wall Load Above
3 - Uniform (PSF)	2' 6" to 20' 6" (Top)	2'	15.8	25.0	Roof Load

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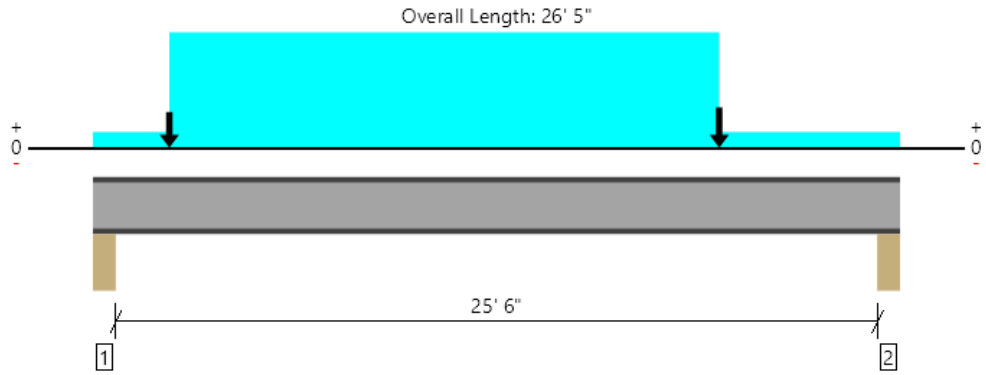
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

2nd Floor, 2B-6 (Steel Opt)
1 piece(s) W10X45 (A992) ASTM Steel



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	14666 @ 4"	31980 (5.50")	Passed (46%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	14632 @ 5 1/2"	70700	Passed (21%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	66639 @ 18' 1 11/16"	88776	Passed (75%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.547 @ 13' 9 1/4"	0.644	Passed (L/565)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	1.141 @ 13' 7 11/16"	1.288	Passed (L/271)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Applicable calculations are based on ANSI/AISC 360-16.
- A lateral-torsional buckling factor (Cb) of 1.0 has been assumed.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Trimmer - HF	5.50"	5.50"	5.50"	7438	5530	4107	14666	None
2 - Trimmer - HF	5.50"	5.50"	5.50"	5938	4866	3256	12029	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 26' 5"	N/A	45.0	--	--	
1 - Uniform (PSF)	2' 6" to 20' 6"	1'	12.0	40.0	-	Floor Load
2 - Uniform (PSF)	0 to 26' 5"	1'	12.0	-	25.0	Low Roof Load
3 - Uniform (PLF)	2' 6" to 20' 6"	N/A	100.0	-	-	Wall Load Above
4 - Uniform (PSF)	2' 6" to 20' 6"	2'	15.8	-	25.0	Roof Load
5 - Point (lb)	2' 6"	N/A	4589	4327	2874	Linked from: 2B-1, Support 1
6 - Point (lb)	20' 6"	N/A	4696	5349	2929	Linked from: 2B-5, Support 2

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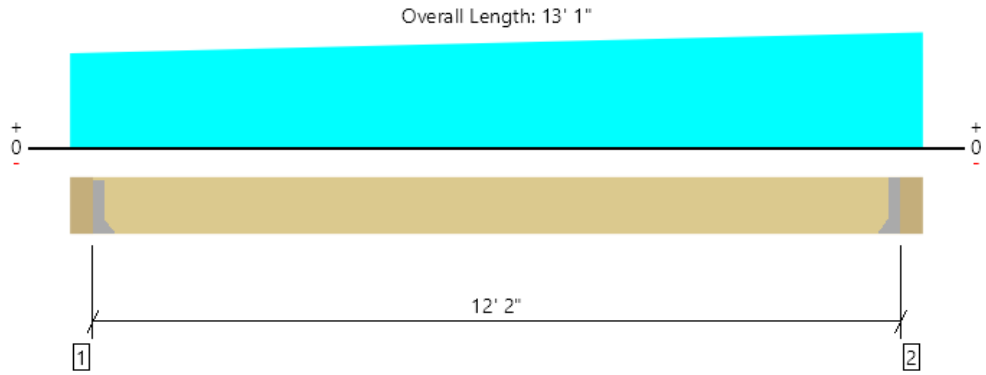
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

2nd Floor, 2B-7

1 piece(s) 5 1/4" x 9 1/2" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4366 @ 12' 7 1/2"	4922 (1.50")	Passed (89%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	3350 @ 11' 10"	9643	Passed (35%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	11582 @ 6' 7 1/16"	19585	Passed (59%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.236 @ 6' 6 3/4"	0.304	Passed (L/620)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.443 @ 6' 6 3/4"	0.608	Passed (L/330)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Hanger on 9 1/2" PSL beam	5.50"	Hanger[1]	1.50"	2050	1963	1149	4384	See note [1]
2 - Hanger on 9 1/2" PSL beam	5.50"	Hanger[1]	1.50"	2197	1963	1382	4706	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	12' 2" o/c	
Bottom Edge (Lu)	12' 2" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	HHUS5.50/10	3.00"	N/A	30-10d	10-10d	
2 - Face Mount Hanger	HHUS5.50/10	3.00"	N/A	30-10d	10-10d	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	5 1/2" to 12' 7 1/2"	N/A	15.6	--	--	
1 - Uniform (PSF)	0 to 13' 1" (Front)	7' 6"	12.0	40.0	-	Floor Load
2 - Uniform (PSF)	0 to 13' 1" (Back)	2' 9"	15.0	-	25.0	Low Roof Load
3 - Uniform (PLF)	0 to 13' 1" (Top)	N/A	100.0	-	-	Wall Load Above
4 - Tapered (PSF)	0 to 13' 1" (Top)	3' to 7'	15.8	-	25.0	Roof Load

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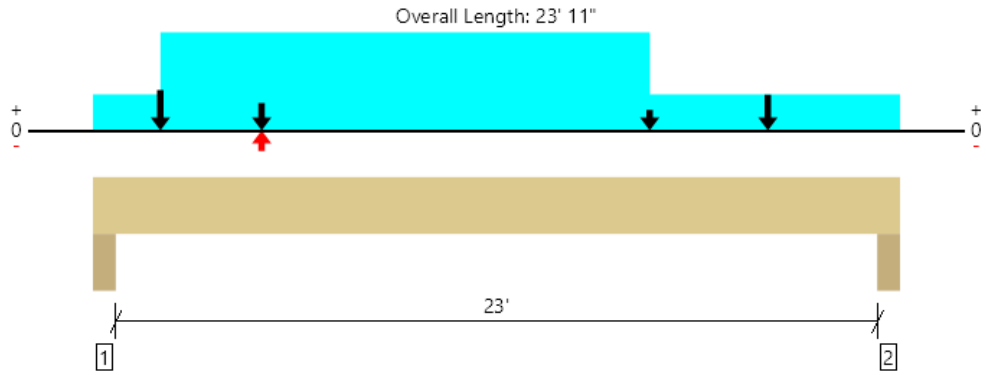
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-8

1 piece(s) 7" x 20" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	17393 @ 4"	24063 (5.50")	Passed (72%)	--	1.0 D - 0.525 E + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	14747 @ 2' 1 1/2"	27067	Passed (54%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	49252 @ 11'	106561	Passed (46%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.291 @ 12' 1 5/8"	0.581	Passed (L/958)	--	1.0 D + 0.525 E + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.570 @ 11' 10 5/8"	1.163	Passed (L/490)	--	1.0 D + 0.525 E + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Member should be side-loaded from both sides of the member or braced to prevent rotation.

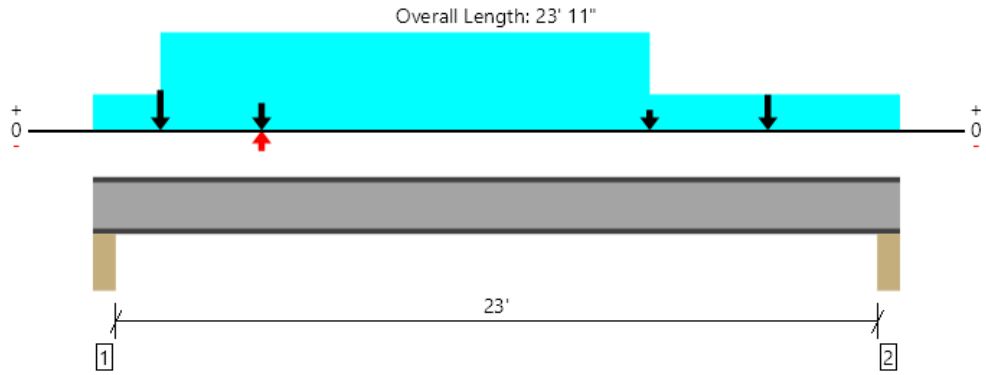
Supports	Bearing Length			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Seismic	Factored	
1 - Trimmer - HF	5.50"	5.50"	3.98"	8333	7415	4084	831/-831	17393	None
2 - Trimmer - HF	5.50"	5.50"	2.55"	4694	6449	1467	831/-831	11143	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	23' 11" o/c	
Bottom Edge (Lu)	23' 11" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Seismic (1.60)	Comments
0 - Self Weight (PLF)	0 to 23' 11"	N/A	43.8	--	--	--	
1 - Uniform (PSF)	0 to 23' 11" (Front)	2'	12.0	40.0	-	-	Floor Load
2 - Uniform (PLF)	2' to 16' 6" (Top)	N/A	100.0	-	-	-	Wall Load Above
3 - Uniform (PSF)	2' to 16' 6" (Top)	2'	15.0	-	25.0	-	Roof Load
4 - Point (lb)	16' 6" (Top)	N/A	-	-	-	1680	EQ = 1.40k x 1.2 overstrength
5 - Point (lb)	5' (Top)	N/A	-	-	-	-1680	EQ = 1.40k x 1.2 overstrength
6 - Point (lb)	5' (Back)	N/A	2050	1963	1149	-	Linked from: 2B-7, Support 1
7 - Point (lb)	2' (Front)	N/A	4612	4327	2779	-	Linked from: 2B-1, Support 2
8 - Point (lb)	20' (Front)	N/A	2860	5660	898	-	Linked from: 2B-5, Support 1

2nd Floor, 2B-8 (Steel Opt)
1 piece(s) W10X45 (A992) ASTM Steel



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	17408 @ 4"	31980 (5.50")	Passed (54%)	--	1.0 D - 0.525 E + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	17349 @ 5 1/2"	70700	Passed (25%)	--	1.0 D - 0.525 E + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	51357 @ 11' 6 11/16"	95237	Passed (54%)	--	1.0 D + 0.525 E + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.388 @ 12' 1 5/8"	0.581	Passed (L/719)	--	1.0 D + 0.525 E + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.760 @ 11' 10 5/8"	1.163	Passed (L/367)	--	1.0 D + 0.525 E + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Applicable calculations are based on ANSI/AISC 360-16.
- A lateral-torsional buckling factor (Cb) of 1.0 has been assumed.

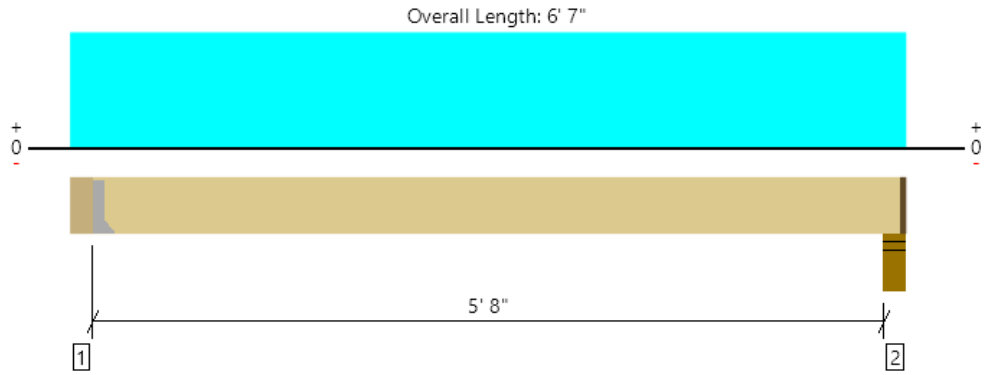
Supports	Bearing Length			Loads to Supports (lbs)					Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Seismic	Factored	
1 - Trimmer - HF	5.50"	5.50"	5.50"	8348	7415	4084	831/-831	17408	None
2 - Trimmer - HF	5.50"	5.50"	5.50"	4709	6449	1467	831/-831	11158	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Seismic (1.60)	Comments
0 - Self Weight (PLF)	0 to 23' 11"	N/A	45.0	--	--	--	
1 - Uniform (PSF)	0 to 23' 11"	2'	12.0	40.0	-	-	Floor Load
2 - Uniform (PLF)	2' to 16' 6"	N/A	100.0	-	-	-	Wall Load Above
3 - Uniform (PSF)	2' to 16' 6"	2'	15.0	-	25.0	-	Roof Load
4 - Point (lb)	16' 6"	N/A	-	-	-	1680	EQ = 1.40k x 1.2 overstrength
5 - Point (lb)	5'	N/A	-	-	-	-1680	EQ = 1.40k x 1.2 overstrength
6 - Point (lb)	5'	N/A	2050	1963	1149	-	Linked from: 2B-7, Support 1
7 - Point (lb)	2'	N/A	4612	4327	2779	-	Linked from: 2B-1, Support 2
8 - Point (lb)	20'	N/A	2860	5660	898	-	Linked from: 2B-5, Support 1

2nd Floor, 2B-9

1 piece(s) 3 1/2" x 9 1/2" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2232 @ 5 1/2"	3281 (1.50")	Passed (68%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	1622 @ 1' 3"	7393	Passed (22%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	3232 @ 3' 4 1/4"	15016	Passed (22%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.023 @ 3' 4 1/4"	0.145	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.046 @ 3' 4 1/4"	0.290	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Hanger on 9 1/2" HF beam	5.50"	Hanger[1]	1.50"	1253	637	1132	2580	See note [1]
2 - Stud wall - HF	5.50"	4.00"	1.69"	1210	614	1090	2488	1 1/2" Rim Board

- Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	6' o/c	
Bottom Edge (Lu)	6' o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Face Mount Hanger	HHUS48	3.00"	N/A	22-10d	8-10d	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	5 1/2" to 6' 5 1/2"	N/A	10.4	--	--	
1 - Uniform (PSF)	0 to 6' 7" (Back)	4' 9"	12.0	40.0	-	Floor Load
2 - Uniform (PSF)	0 to 6' 7" (Front)	1' 6"	12.0	-	25.0	Low Roof Load
3 - Uniform (PLF)	0 to 6' 7" (Top)	N/A	100.0	-	-	Wall Load Above
4 - Uniform (PSF)	0 to 6' 7" (Top)	12'	15.8	-	25.0	Roof Load

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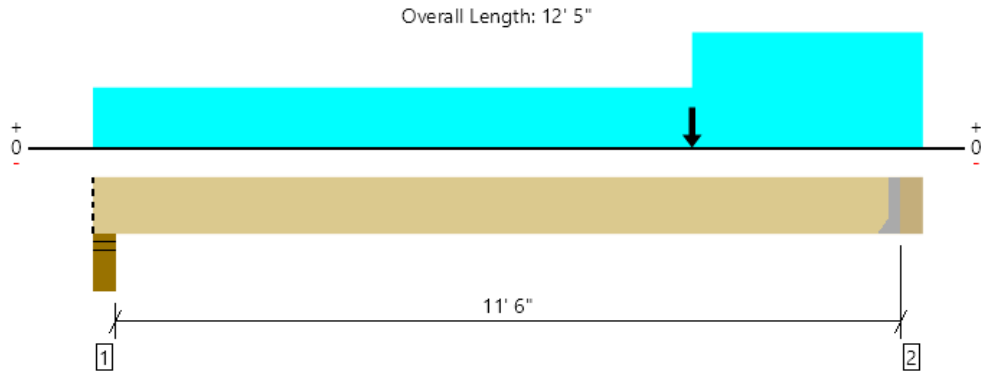
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-10

1 piece(s) 5 1/4" x 9 1/2" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	4850 @ 11' 11 1/2"	4922 (1.50")	Passed (99%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	3953 @ 11' 2"	9643	Passed (41%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	10911 @ 7' 5 5/16"	19585	Passed (56%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.207 @ 6' 4 5/8"	0.291	Passed (L/675)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.347 @ 6' 5 1/4"	0.581	Passed (L/402)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Stud wall - HF	5.50"	5.50"	1.51"	1084	2129	354	3213	Blocking
2 - Hanger on 9 1/2" PSL beam	5.50"	Hanger[1]	1.50"	2267	2482	1376	5160	See note [1]

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	12' o/c	
Bottom Edge (Lu)	12' o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
2 - Face Mount Hanger	HHUS5.50/10	3.00"	N/A	30-10d	10-10d	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 11' 11 1/2"	N/A	15.6	--	--	
1 - Uniform (PSF)	0 to 12' 5" (Front)	8'	12.0	40.0	-	Floor Load
2 - Uniform (PLF)	9' to 12' 5" (Top)	N/A	100.0	-	-	Wall Load Above
3 - Uniform (PSF)	9' to 12' 5" (Top)	7'	15.8	-	25.0	Roof Load
4 - Point (lb)	9' (Back)	N/A	1253	637	1132	Linked from: 2B-9, Support 1

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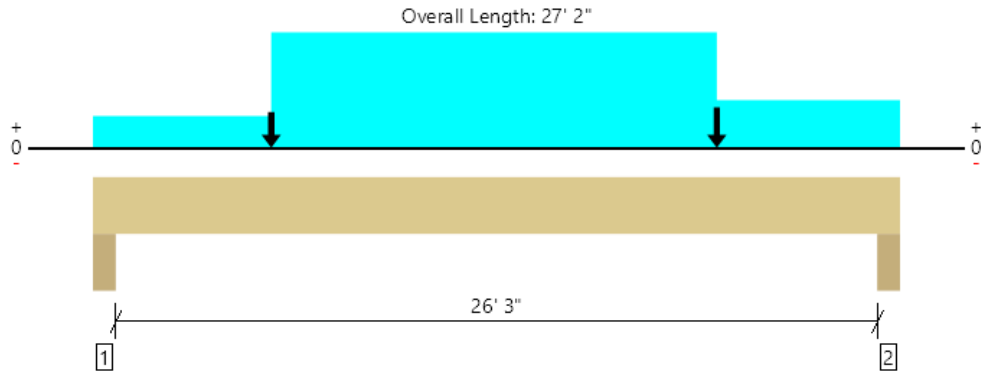
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-11

1 piece(s) 5 1/4" x 20" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	7814 @ 26' 10"	18047 (5.50")	Passed (43%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	7548 @ 25' 1/2"	23345	Passed (32%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	49698 @ 14' 1 5/16"	91909	Passed (54%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.421 @ 13' 8 5/16"	0.663	Passed (L/755)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.916 @ 13' 7 3/4"	1.325	Passed (L/347)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Trimmer - HF	5.50"	5.50"	2.28"	3987	2391	2273	7485	None
2 - Trimmer - HF	5.50"	5.50"	2.38"	4045	2654	2372	7814	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	27' 2" o/c	
Bottom Edge (Lu)	27' 2" o/c	

•Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 27' 2"	N/A	32.8	--	--	
1 - Uniform (PSF)	0 to 6' (Front)	2'	12.0	-	25.0	Low Roof Load
2 - Uniform (PSF)	21' to 27' 2" (Front)	3'	12.0	-	25.0	Low Roof Load
3 - Uniform (PSF)	6' to 21' (Front)	1'	12.0	40.0	-	Floor Load
4 - Uniform (PSF)	6' to 21' (Back)	1'	12.0	-	25.0	Low Roof Load
5 - Uniform (PLF)	6' to 21' (Top)	N/A	100.0	-	-	Wall Load Above
6 - Uniform (PSF)	6' to 21' (Top)	2'	15.0	-	25.0	Roof Load
7 - Point (lb)	6' (Front)	N/A	2197	1963	1382	Linked from: 2B-7, Support 2
8 - Point (lb)	21' (Front)	N/A	2267	2482	1376	Linked from: 2B-10, Support 2

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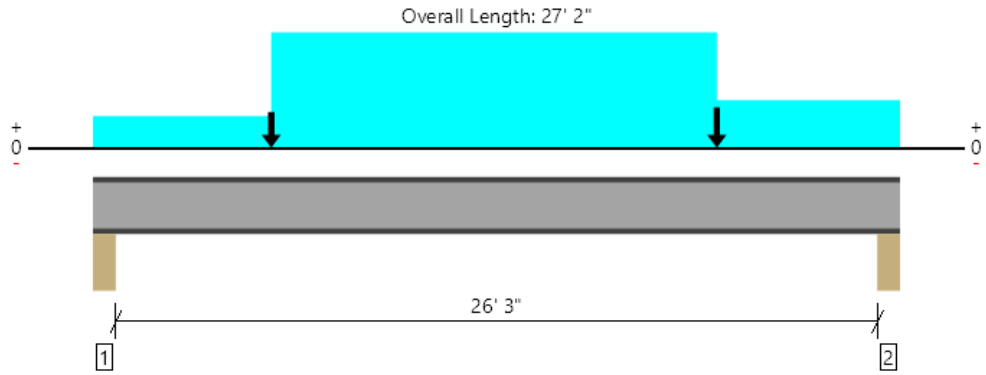
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

2nd Floor, 2B-11 (Steel Opt)
1 piece(s) W12X40 (A992) ASTM Steel



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	7912 @ 26' 10"	31940 (5.50")	Passed (25%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	7851 @ 26' 8 1/2"	70210	Passed (11%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Moment (Ft-lbs)	50328 @ 14' 1 1/8"	66387	Passed (76%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.345 @ 13' 8 5/16"	0.663	Passed (L/922)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.758 @ 13' 7 3/4"	1.325	Passed (L/420)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Applicable calculations are based on ANSI/AISC 360-16.
- A lateral-torsional buckling factor (Cb) of 1.0 has been assumed.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Trimmer - HF	5.50"	5.50"	5.50"	4084	2391	2273	7583	None
2 - Trimmer - HF	5.50"	5.50"	5.50"	4142	2654	2372	7912	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 27' 2"	N/A	40.0	--	--	
1 - Uniform (PSF)	0 to 6'	2'	12.0	-	25.0	Low Roof Load
2 - Uniform (PSF)	21' to 27' 2"	3'	12.0	-	25.0	Low Roof Load
3 - Uniform (PSF)	6' to 21'	1'	12.0	40.0	-	Floor Load
4 - Uniform (PSF)	6' to 21'	1'	12.0	-	25.0	Low Roof Load
5 - Uniform (PLF)	6' to 21'	N/A	100.0	-	-	Wall Load Above
6 - Uniform (PSF)	6' to 21'	2'	15.0	-	25.0	Roof Load
7 - Point (lb)	6'	N/A	2197	1963	1382	Linked from: 2B-7, Support 2
8 - Point (lb)	21'	N/A	2267	2482	1376	Linked from: 2B-10, Support 2

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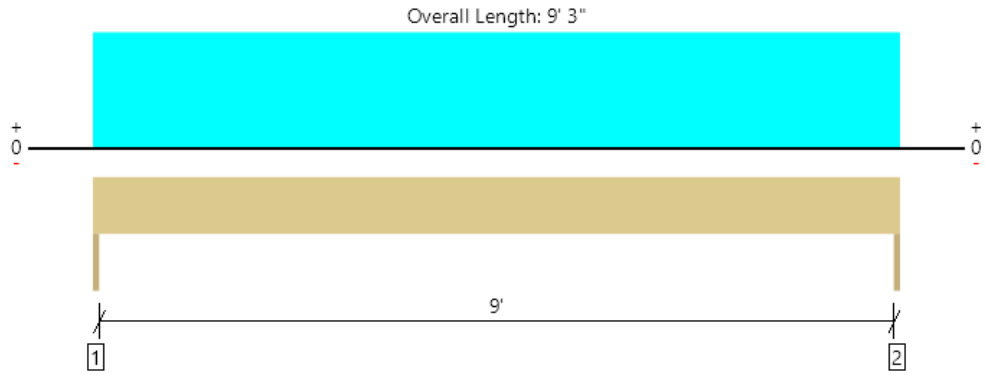
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

1st Floor, 1H-1 (Garage Header)
1 piece(s) 3 1/2" x 9 1/2" 2.2E Parallam® PSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	390 @ 0	3281 (1.50")	Passed (12%)	--	1.0 D + 1.0 S (All Spans)
Shear (lbs)	313 @ 11"	7393	Passed (4%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	903 @ 4' 7 1/2"	14529	Passed (6%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.017 @ 4' 7 1/2"	0.308	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.028 @ 4' 7 1/2"	0.463	Passed (L/999+)	--	1.0 D + 1.0 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- A 3.2% decrease in the moment capacity has been added to account for lateral stability.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Snow	Factored	
1 - Trimmer - HF	1.50"	1.50"	1.50"	159	231	390	None
2 - Trimmer - HF	1.50"	1.50"	1.50"	159	231	390	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 9' 3"	N/A	10.4	--	
1 - Uniform (PSF)	0 to 9' 3"	2'	12.0	25.0	Low Roof Load

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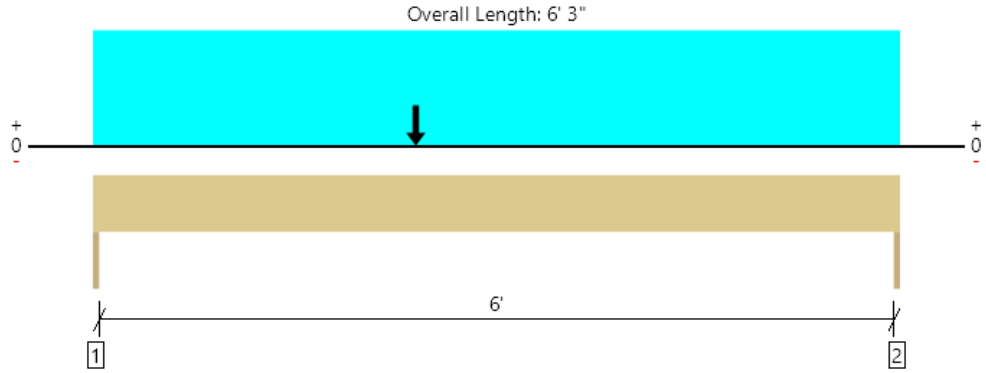
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

1st Floor, 1H-2

1 piece(s) 3 1/2" x 9" 24F-V4 DF Glulam



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2982 @ 0	3413 (1.50")	Passed (87%)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Shear (lbs)	2867 @ 10 1/2"	6400	Passed (45%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Pos Moment (Ft-lbs)	7042 @ 2' 6"	10679	Passed (66%)	1.15	1.0 D + 0.75 L + 0.75 S (All Spans)
Live Load Defl. (in)	0.054 @ 3' 3/16"	0.208	Passed (L/999+)	--	1.0 D + 0.75 L + 0.75 S (All Spans)
Total Load Defl. (in)	0.105 @ 3' 3/16"	0.313	Passed (L/714)	--	1.0 D + 0.75 L + 0.75 S (All Spans)

System : Wall
Member Type : Header
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- A 1.7% decrease in the moment capacity has been added to account for lateral stability.
- Critical positive moment adjusted by a volume factor of 1.00 that was calculated using length L = 6' 3".
- The effects of positive or negative camber have not been accounted for when calculating deflection.
- The specified glulam is assumed to have its strong laminations at the bottom of the beam. Install with proper side up as indicated by the manufacturer.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)				Accessories
	Total	Available	Required	Dead	Floor Live	Snow	Factored	
1 - Trimmer - HF	1.50"	1.50"	1.50"	1454	575	1463	2982	None
2 - Trimmer - HF	1.50"	1.50"	1.50"	1029	383	1080	2126	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	End Bearing Points	
Bottom Edge (Lu)	End Bearing Points	

Vertical Loads	Location	Tributary Width	Dead (0.90)	Floor Live (1.00)	Snow (1.15)	Comments
0 - Self Weight (PLF)	0 to 6' 3"	N/A	7.7	--	--	
1 - Uniform (PSF)	0 to 6' 3"	4'	12.4	-	25.0	Low Roof Load
2 - Point (lb)	2' 6"	N/A	2125	958	1918	Linked from: 2B-3, Support 2

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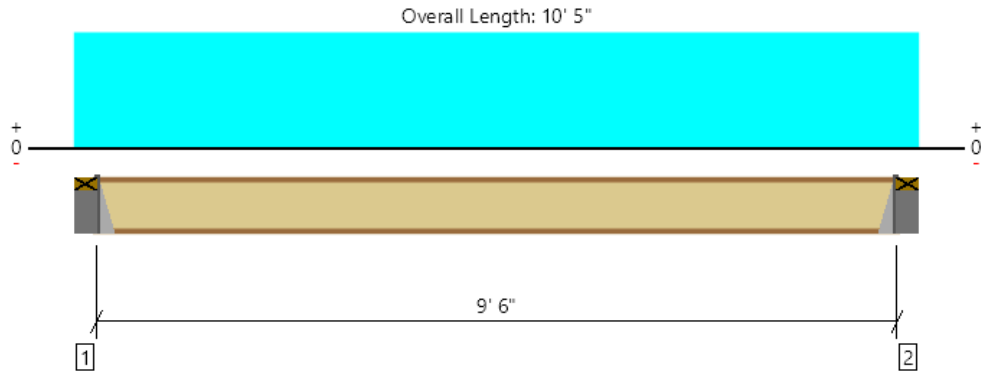
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

1st Floor, 1J-1 (Under Exercise Room)
1 piece(s) 9 1/2" TJI® 210 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	538 @ 5 1/2"	1005 (1.75")	Passed (54%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	538 @ 5 1/2"	1330	Passed (40%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1279 @ 5' 2 1/2"	3000	Passed (43%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.083 @ 5' 2 1/2"	0.237	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.118 @ 5' 2 1/2"	0.475	Passed (L/967)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	60	40	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: None.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Hanger on Single 2X DF plate	5.50"	Hanger[1]	1.75" / - [2]	174	417	590	See note [1]
2 - Hanger on Single 2X DF plate	5.50"	Hanger[1]	1.75" / - [2]	174	417	590	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.
- [2] Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	5' 9" o/c	
Bottom Edge (Lu)	9' 6" o/c	

- TJI joists are only analyzed using Maximum Allowable bracing solutions.
- Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Top Mount Hanger	ITS2.06/9.5	2.00"	4-10dx1.5	2-10dx1.5	2-Strong-Grip	
2 - Top Mount Hanger	ITS2.06/9.5	2.00"	4-10dx1.5	2-10dx1.5	2-Strong-Grip	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 10' 5"	16"	25.0	60.0	Exercise Loading

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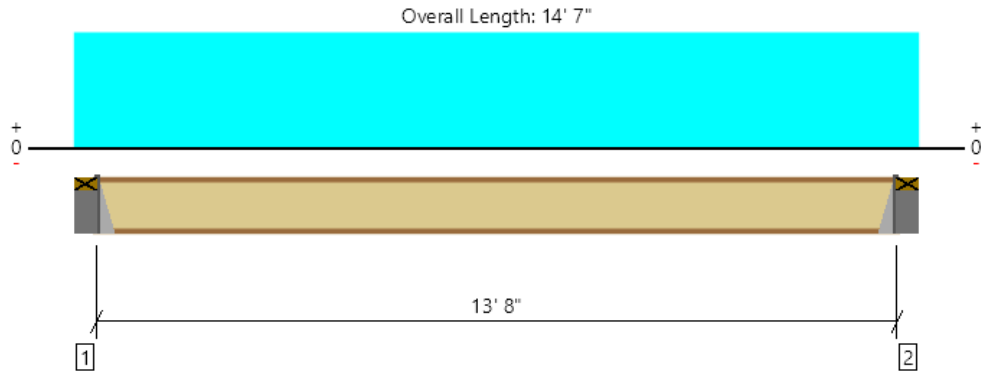
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

1st Floor, 1J-2
1 piece(s) 9 1/2" TJI® 210 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	474 @ 5 1/2"	1005 (1.75")	Passed (47%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	474 @ 5 1/2"	1330	Passed (36%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1619 @ 7' 3 1/2"	3000	Passed (54%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.208 @ 7' 3 1/2"	0.342	Passed (L/789)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.270 @ 7' 3 1/2"	0.683	Passed (L/607)	--	1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	44	40	Passed	--	--

System : Floor
Member Type : Joist
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: None.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Hanger on Single 2X DF plate	5.50"	Hanger[1]	1.75" / - [2]	117	389	506	See note [1]
2 - Hanger on Single 2X DF plate	5.50"	Hanger[1]	1.75" / - [2]	117	389	506	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.
- [2] Required Bearing Length / Required Bearing Length with Web Stiffeners

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	5' 1" o/c	
Bottom Edge (Lu)	13' 8" o/c	

- TJI joists are only analyzed using Maximum Allowable bracing solutions.
- Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Top Mount Hanger	ITS2.06/9.5	2.00"	4-10dx1.5	2-10dx1.5	2-Strong-Grip	
2 - Top Mount Hanger	ITS2.06/9.5	2.00"	4-10dx1.5	2-10dx1.5	2-Strong-Grip	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Load	Location	Spacing	Dead (0.90)	Floor Live (1.00)	Comments
1 - Uniform (PSF)	0 to 14' 7"	16"	12.0	40.0	Floor Loading

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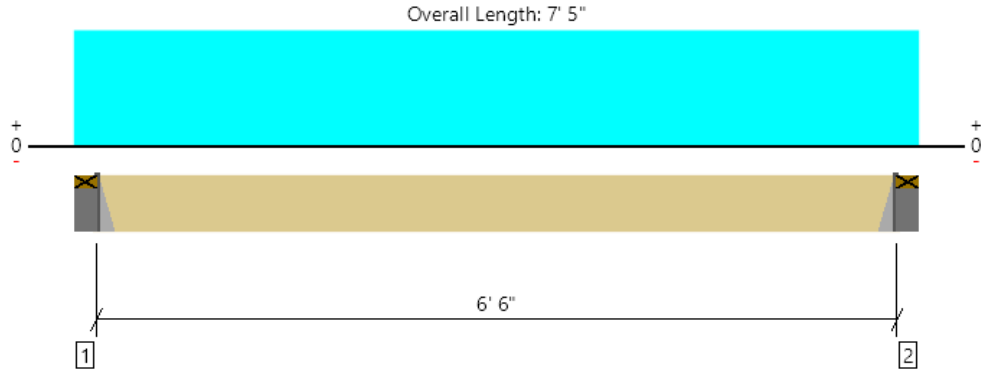
The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

1st Floor, 1B-1
1 piece(s) 4 x 8 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	866 @ 5 1/2"	3281 (1.50")	Passed (26%)	--	1.0 D + 1.0 L (All Spans)
Shear (lbs)	705 @ 1' 3/4"	3045	Passed (23%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	1407 @ 3' 8 1/2"	2989	Passed (47%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.045 @ 3' 8 1/2"	0.162	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.060 @ 3' 8 1/2"	0.325	Passed (L/999+)	--	1.0 D + 1.0 L (All Spans)

System : Floor
Member Type : Flush Beam
Building Use : Residential
Building Code : IBC 2018
Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

Supports	Bearing Length			Loads to Supports (lbs)			Accessories
	Total	Available	Required	Dead	Floor Live	Factored	
1 - Hanger on Single 2X HF plate	5.50"	Hanger[1]	1.50"	243	742	985	See note [1]
2 - Hanger on Single 2X HF plate	5.50"	Hanger[1]	1.50"	243	742	985	See note [1]

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- [1] See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	6' 6" o/c	
Bottom Edge (Lu)	6' 6" o/c	

•Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie						
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories
1 - Top Mount Hanger	BA3.56/7.25	3.00"	6-10dx1.5	4-10dx1.5	2-10dx1.5	
2 - Top Mount Hanger	BA3.56/7.25	3.00"	6-10dx1.5	4-10dx1.5	2-10dx1.5	

- Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
0 - Self Weight (PLF)	5 1/2" to 6' 11 1/2"	N/A	6.4	--	
1 - Uniform (PSF)	0 to 7' 5" (Front)	5'	12.0	40.0	Floor Load

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ForteWEB Software Operator	Job Notes
Harrison Kliegl L120 Engineering (425) 636-3313 hkliegl@l120engineering.com	

Weyerhaeuser

LONGITUDE
ONE TWENTY°
ENGINEERING & DESIGN

LATERAL CALCULATIONS

SHEAR WALL REFERENCE PER PLAN

(425) 636 3313
L120Engineering.com

Project Number:	Plan Name:	Sheet Number:
S221118-2	Litchfield Residence	DC
Engineer:	Specifics:	Date:
HK	Design Criteria	4/2/2023

Gravity Criteria: BLUE = Review and update as required - Typical Input Code: IBC 2018

ROOF SYSTEM		
Live Load:		
Snow	25.0	psf
Dead Load:		
Composite Roofing	2.0	psf
19/32" Plywood Sheathing	2.5	psf
Trusses at 24" o.c.	3.0	psf
Insulation	1.8	psf
(2) Layers 5/8" GWB	4.4	psf
Misc or Tile Roof	1.3	psf
Total	**15.0**	**psf**

FLOOR SYSTEM		
Live Load:		
Residential	40.0	psf
Dead Load:		
Flooring	3.0	psf
3/4" T & G Plywood	2.5	psf
Floor Joists at 16" o.c.	2.5	psf
Insulation	0.5	psf
(1) Layers 5/8" GWB	2.2	psf
Misc or Tile Flooring	1.3	psf
Total	**12.0**	**psf**

EXTERIOR WALL SYSTEM		
2x6 at 16" o.c.	1.7	psf
Insulation	1.0	psf
1/2" Plywood Sheathing	1.5	psf
(2) layers 5/8" GWB	4.4	psf
Misc or Brick Covered Wall	3.4	psf
Total	**12.0**	**psf**

INTERIOR WALL SYSTEM		
2x4 at 16" o.c.	1.1	psf
Insulation	0.5	psf
(2) Layers 5/8" GWB	4.4	psf
Misc	2.0	psf
Total	**8.0**	**psf**

SEISMIC PARAMETERS:

Code Reference: ASCE 7-16
R = 6.5 Bearing Wall System, Wood Structural Panel Walls
Mapped Spectral Acceleration, Ss = 1.6
Mapped Spectral Acceleration, S1 = 0.63
Soil Site Class = D

WIND PARAMETERS:

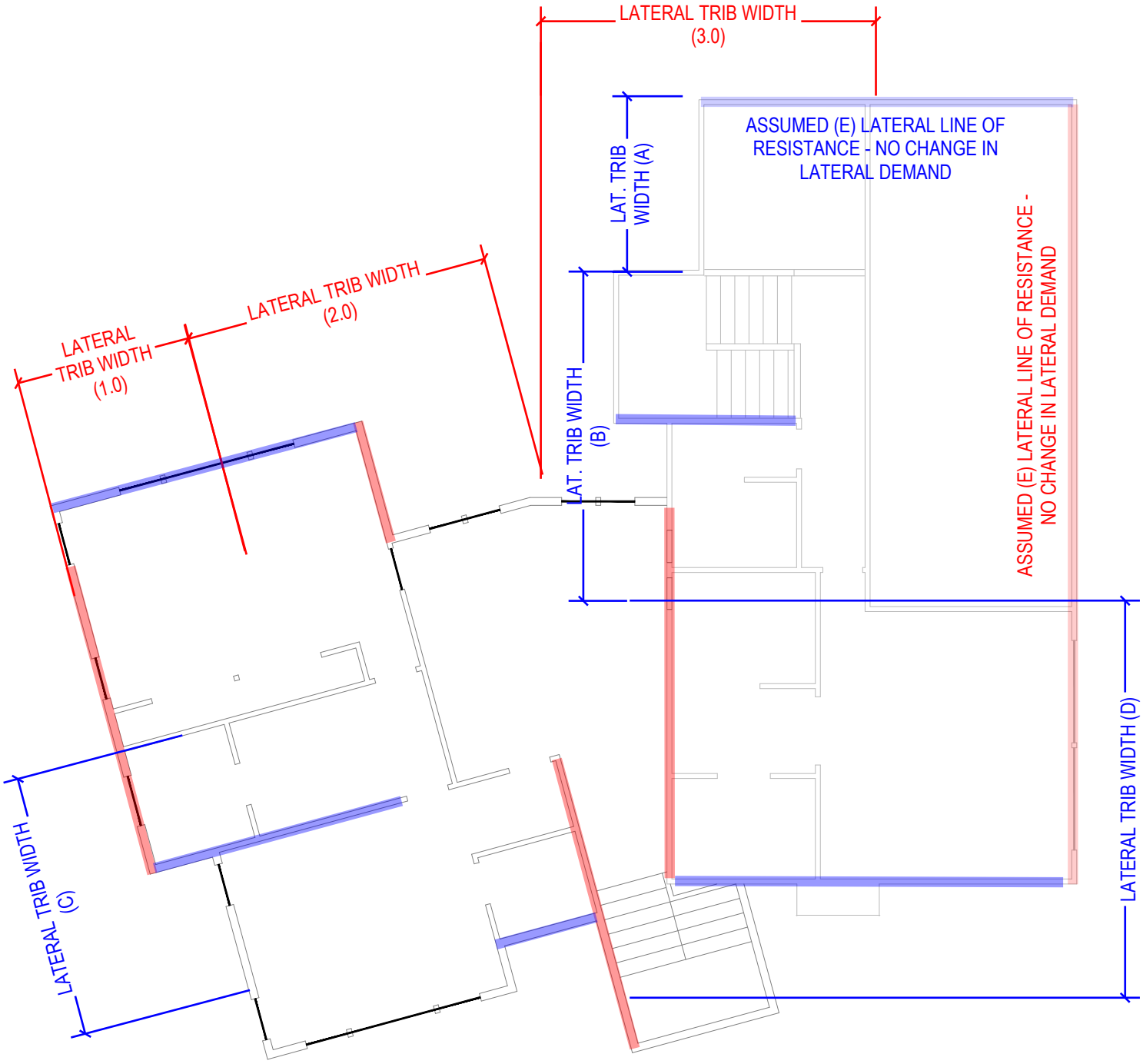
Code Reference: ASCE 7-16
Basic Wind Speed (3 second Gust) = 100 mph
Exposure : B
Kzt = 1.60

SOIL PARAMETERS:

Soil Bearing Pressure = 1,500 psf competent native soil or structural fill
1/3 increase for short-term wind or seismic loading is acceptable
Frost Depth = 18 in

Lateral Wall Pressures:
Unrestrained Active Pressure = 35 pcf Cantilevered walls
Restrained Active Pressure = 50 pcf Plate Wall Design/Tank Walls
Passive Pressure = 250 pcf
Soil Friction Coeff. = 0.35

LONGITUDE
ONE TWENTY°
ENGINEERING & DESIGN



UPPER FLOOR LATERAL TRIB DISTRIBUTION

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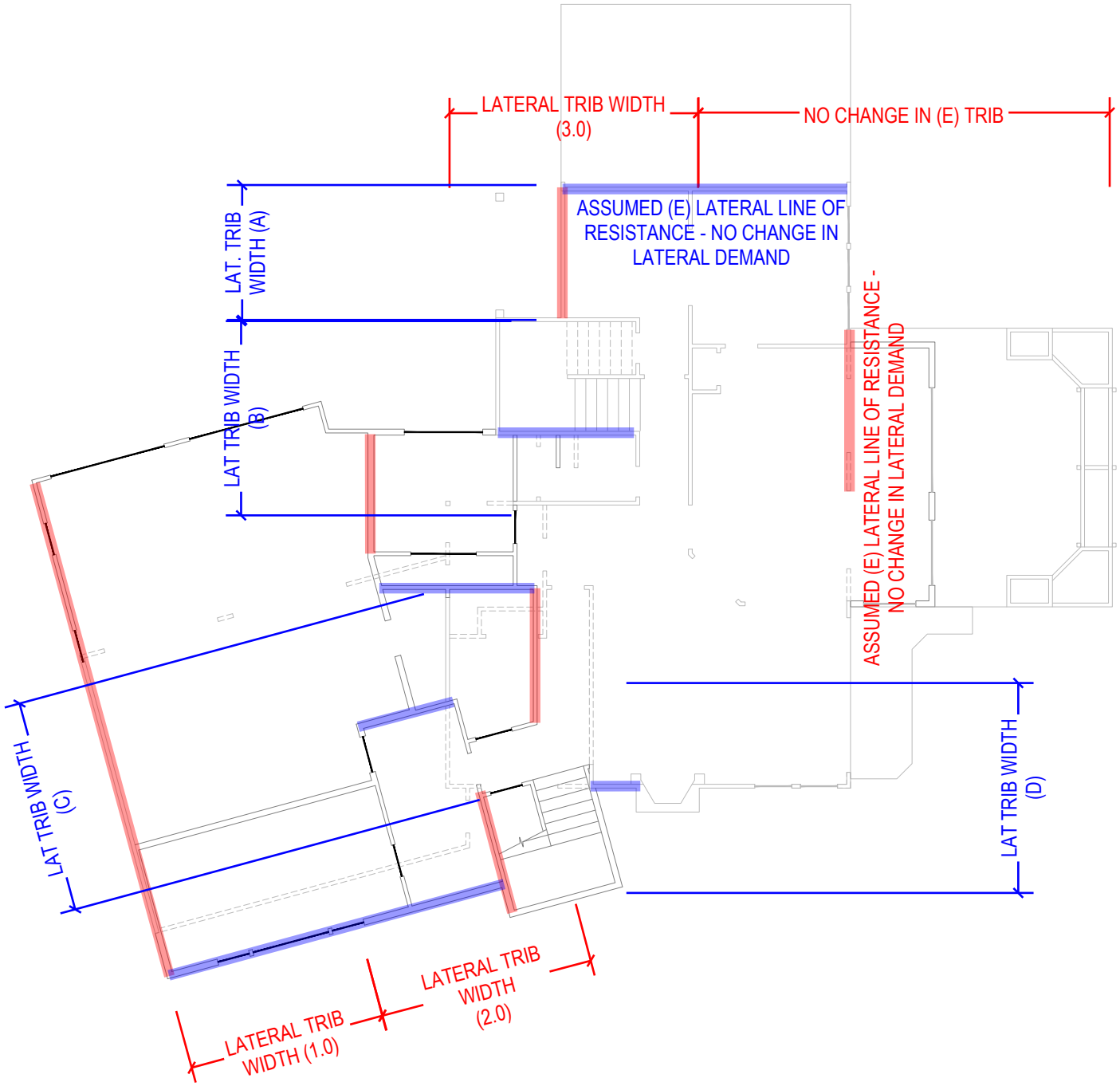

MAIN FLOOR LATERAL TRIB DISTRIBUTION

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Project Number:	Plan:	Sheet Number:
S221118-2	**Litchfield Residence**	**L1**
Engineer:	**Specifics:**	**Date**
HK	**WIND FORCES**	**4/2/2023**

IBC 2018 Section 1609 → ASCE 7-16 Section 28.6 - Simplified Procedure → Main Wind-Force Resisting System

WIND DESIGN CRITERIA:

Basic Wind Speed, V_s =	100	mph	(ASCE 7-16, Section 26.5)
Exposure =	B		(ASCE 7-16, Section 26.7)

BUILDING DIMENSIONS:

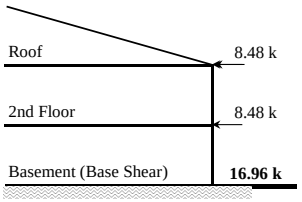
Roof Slope =	5.00	:12	=	22.62	degrees
Loads From Front/Back - Width (ft)=	49.00	ft		Roof:	Gable
Loads From Side - Width (ft) =	45.00	ft		Roof:	Gable
Average Eave Height =	20.00	ft			
Mean Roof Ht. , h =	27.00	ft	(ASCE 7-16, Figure 27.6-2)		
Edge Strip Width, a =	4.5	ft	(ASCE 7-16, Figure 28.6-1)		
End Zone Width, 2a =	9.00	ft	(ASCE 7-16, Figure 28.6-1)		

TOPOGRAPHIC DESIGN CONSIDERATIONS:

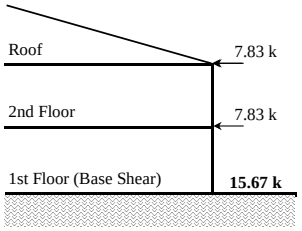
Topographic Factor , Kzt =	**1.60**	(ASCE 7-16, Section 26.8)
Adjustment Factor, λ =	1.00	(ASCE 7-16, Figure 28.6-1)

WIND LOAD SUMMARY:

Front / Back Direction



Side / Side Direction

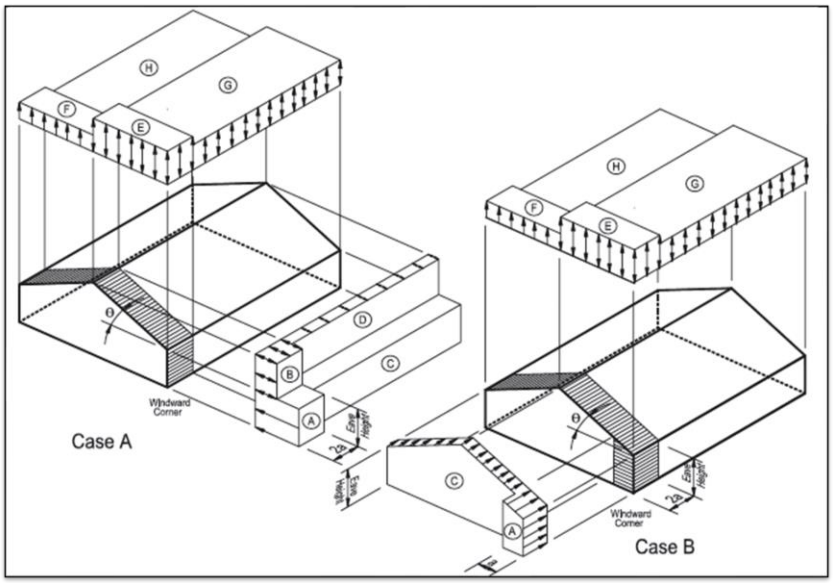


SIMPLIFIED DESIGN WIND PRESSURE, P_{S30} (psf)

(Exposure B at h = 30ft.)

Basic Wind Speed, Vs (mph)	Roof Angle (Degrees)	Load Case	ZONES*									
			Horizontal Pressure				Vertical Presssure				Overhang	
			A	B	C	D	E	F	G	H	E_{OH}	G_{OH}
100	22.62	A	19.90	3.20	14.40	3.30	-8.80	-12.00	-6.40	-9.70	-16.50	-14.00

** Values Interpolated from Figure 28.6-1 ASCE 7 - 16*



Project Number:	Plan:	Sheet Number:
S221118-2	**Litchfield Residence**	**L1**
Engineer:	Specifics:	Date
HK	**WIND FORCES**	**4/2/2023**

IBC 2018 Section 1609 → ASCE 7-16 Section 28.6 - Simplified Procedure → Main Wind-Force Resisting System

HORIZONTAL LOADS (psf) $p_s = \lambda * Kzt * Ps30$				**MIN. LOADS (psf)** Per ASCE 7-16, 28.6.3	
End zone		Interior zone		Roof	Wall
A (Wall)	B (Roof)	C (Wall)	D (Roof)		
31.84	5.12	23.04	5.28	8.0	16.0

ASD WIND FORCES: FRONT / BACK LOADING DIRECTION

	Location	Width (ft)	Height (ft)	Plane	End Zone Length (ft)	End Zone Pressure (W) (psf)	Interior zone Length (ft)	Interior zone Pressure (W) (psf)	Force 0.6 ω*W (kips)	Min Force 0.6 ω*W (kips)
ROOF	"Height" of Roof to Plate (see note)	49.0	4.50	(roof)	9.0	31.84	40.0	23.04	4.24	1.38
ROOF	Plate to Mid 2nd LVL	49.0	4.50	(wall)	9.0	31.84	40.0	23.04	4.24	2.75
								Σ =	8.48	4.13
2nd FLOOR	Mid 2nd LVL to Floor	49.0	4.50	(wall)	9.0	31.84	40.0	23.04	4.24	2.75
2nd FLOOR	"Height" Low-Roof to Plate (see note)	0.0	0.00	(roof)	9.0	31.84	-9.0	23.04	0.00	0.00
2nd FLOOR	Floor to Mid 1st LVL	49.0	4.50	(wall)	9.0	31.84	40.0	23.04	4.24	2.75
								Σ =	8.48	5.50
								Total Wind Base Shear (kips)	16.96	9.63

ASD WIND FORCES: SIDE / SIDE LOADING DIRECTION

	Location	Width (ft)	Height (ft)	Plane	End Zone Length (ft)	End Zone Pressure (W) (psf)	Interior zone Length (ft)	Interior zone Pressure (W) (psf)	Force 0.6 ω*W kips	Min Force 0.6 ω*W kips
ROOF	"Height" of Roof to Plate (see note)	45.0	4.50	(roof)	9.0	31.84	36.0	23.04	3.92	1.26
ROOF	Plate to Mid 2nd LVL	45.0	4.50	(wall)	9.0	31.84	36.0	23.04	3.92	2.53
								Σ =	7.83	3.79
2nd FLOOR	Mid 2nd LVL to Floor	45.0	4.50	(wall)	9.0	31.84	36.0	23.04	3.92	2.53
2nd FLOOR	"Height" Low-Roof to Plate (see note)	0.0	0.00	(roof)	9.0	31.84	-9.0	23.04	0.00	0.00
2nd FLOOR	Floor to Mid 1st LVL	45.0	4.50	(wall)	9.0	31.84	36.0	23.04	3.92	2.53
								Σ =	7.83	5.05
								Total Wind Base Shear (kips)	15.67	8.85

Project Number:	Plan Name:	Sheet Number:
S221118-2	**Litchfield Residence**	**L2**
Engineer:	Specifics:	Date:
HK	**SEISMIC WEIGHTS**	**4/2/2023**

Unit Weights (psf)

Roof:	15	psf	
Floor:	12	psf	
Exterior Wall:	12	psf	
Interior Wall:	8	psf	

Seismic Weights include: (REF §12.7)

- 25% of storage Live loads
- Actual partition weight or 10 psf min if applicable
- Operating weight of permenant equipment
- 20% of uniform design snow loads for areas where Pf > 30 psf

LEVEL	ITEM	AREA / LENGTH	HEIGHT (ft)	UNIT WEIGHT (psf)		Item Total Weight. (lbs)	Level Sub-Total (kips)	Average Pressure (psf)
ROOF								
	Roof	2,500	1.09	15	=	40,777		
	Ext. Wall Below	250	4.00	12	=	12,000		
	Corridor Wall Below	125	4.00	8	=	4,000		
							57	**23**
2nd FLOOR								
	Floor	2,050	1.00	12	=	24,600		
	Low Roof	1,430	1.09	15	=	23,324		
	Ext. Wall Above	250	4.00	12	=	12,000		
	Corridor Wall Above	125	4.00	8	=	4,000		
	Ext. Wall Below	225	4.00	12	=	10,800		
	Corridor Wall Below	110	4.00	8	=	3,520		
							78	**22**
1st FLOOR								
	Ext. Wall Above	225	4.00	12	=	10,800		
	Corridor Wall Above	110	4.00	8	=	3,520		
							14	

STRUCTURE WEIGHT FOR SEISMIC BASE SHEAR: **135** **kips**

TOTAL WEIGHT OF STRUCTURE: **149** **kips**

(Includes Basement Dead Load)

Project Number:	Plan Name:	Sheet Number:
S221118-2	**Litchfield Residence**	**L3**
Engineer:	Specifics:	Date:
HK	**SEISMIC FORCES**	**4/2/2023**

Equivelant Lateral Force Analysis per IBC 2018 1613.1 →ASCE 7-16 Table 12.6-1 →Sec 12.8

Data generated by: Seismic Design Values for Buildings **"Java Ground Motion Parameter Calculation"**

S_1 =	0.63		Maps
S_{DS} =	1.18		(ASCE 7 EQ 11.4.-3)
S_{D1} =	0.87		(ASCE 7 EQ 11.4.-4)
Seismic Importance Factor =	1.00		(ASCE 7 Table 11.5-1)
Seismic Design Category =	D		(ASCE 7 Table 11.6-1 & 11.6.2)
Response Modification Factor, R =	6.5		(ASCE 7 Table 12.2-1)
Seismic Force-Resisting System Description =	A.13 - light framed walls		

Building Height, h_n =	20.0	ft	
Building Period Coefficient, C_T =	0.020		(ASCE 7 Table 12.8.-2)
Approx. Fundamental Period, T_a =	0.189	$(C_{T*}(h_n^{0.75})$	(ASCE 7 EQ 12.8.-7)
Approx. Fundamental Period, T_L =	6.0	sec	(ASCE 7 11.4.5)

Seismic Response Coefficient

$C_s = S_{DS}/(R/I)$	C_s =	0.182		(ASCE 7 EQ 12.8.-2)

Seismic Response Coefficient, Maximum

$C_{s,\,MAX} = S_{D1}/(T*R/I)$	$C_{s,\,MAX}$ =	0.708	$T \le T_L$	(ASCE 7 EQ 12.8.-3)
$C_{s,\,MAX} = S_{D1}\,T_L/(T^2*R/I)$	$C_{s,\,MAX}$ =	NA	$T > T_L$	(ASCE 7 EQ 12.8.-4)

Seismic Response Coefficient, Minimum

$C_{s,\,MIN} = 0.01$	$C_{s,\,MIN}$ =	0.010		(ASCE 7 EQ 12.8.-5)
$C_{s,\,MIN} = 0.5\,S_1 / (R/I)$	$C_{s,\,MIN}$ =	0.048	if S1 > 0.6	(ASCE 7 EQ 12.8.-6)

C_s =	**0.182**		
Dead Load W =	135	kips	
V = Cs W =	24.5	kips	(ASCE 7 EQ 12.8.-1)
$Q_E = V$ =	24.5	kips	(ASCE 7 EQ 12.4-3)
ρ =	1.0		(ASCE 7 12.3.4.2)
$E_H = \rho\,Q_E$ =	24.5	kips	(ASCE 7 EQ 12.4-3)
Ev = .2 S_{DS} D =	0.24	x D kips	

Factor for Alternate Basic Load conbinations - 2018 IBC 1605.3.2

$E_H/1.4$ =	**17.5**	**kips**	IBC 2018 1605.3.2
k =	1		(ASCE 7 12.8.3)

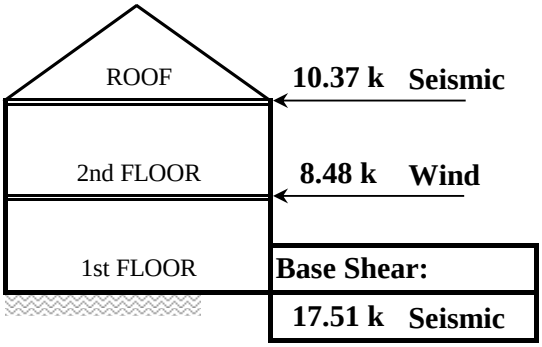
VERTICAL DISTRIBUTION (Per ASCE 7 - 12.8.3)

Floor	Area (ft^2)	Story Height H (ft)	Total Height h_x (ft)	Story Weight w_x (kips)	$w_x h_x^k$ (k-ft)	Vert Dist Factor Cvx	Story Force Fx (kips)	Factored Story Force (ASD) Fx ρ/1.4 = E_H/1.4 (kips)
Roof	2,500	9.00	18.00	57	1,022	0.59	14.5	**10.4**
2nd	2,050	9.00	9.00	78	704	0.41	10.0	**7.1**
				Sum =	1,726	1.000	24.5	**17.5**

Project Number:	Plan Name:	Sheet Number:
S221118-2	**Litchfield Residence**	**L4**
Engineer:	**Specifics:**	**Date:**
HK	**DESIGN LOADS**	**4/2/2023**

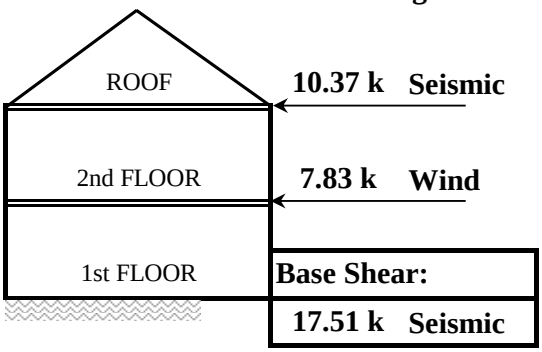
FRONT / BACK APPLIED FORCES

Wind Force $0.6\,\omega * W_S$ (kips)		Seismic Force $E/1.4$ (kips)	
Per Level	Sum	Per Level	Sum
8.48		10.37	
	8.48		10.37
8.48		7.14	
	16.96		17.51



SIDE / SIDE APPLIED FORCES

Wind Force $0.6\,\omega * W_S$ (kips)		Seismic Force $E/1.4$ (kips)	
Per Level	Sum	Per Level	Sum
7.83		10.37	
	7.83		10.37
7.83		7.14	
	15.67		17.51



Project Number:	Plan Name:	Sheet Number:
S221118-2	Litchfield Residence	L5
Engineer:	**Specifics:**	**Date:**
HK	Shear walls	4/2/2023

Notes:

* All walls designed with Force-Transfer should meet a minimum height to width ratio of 2:1 at Pier (SDPWS 2018, Table 4.3.4)
* Maximum allowed height to width ratio 3.5:1 for walls w/o openings (increased shear design values per SDPWS 2018, Table 4.3.4)
* Shear panel height is height to underside or roof or floor framing.

RED = Update Formula as required - Important
BLUE = Review and update as required - Typical Input

2nd Story Walls (Front - Back Direction)

Stud Species: HF

"Adjusted" Story shear(kips) = 10.37
Story height (ft) = 10.00
Shear Panel height (ft) = 9.00
Total Diaphragm Width (ft) = 49.00

100% story shear: YES

Governing Force (F/B Direction) = Seismic
Dead load factor (F/B Direction) = 0.90 IBC 2018 Equation 16-22
Shear panel capacity (Wind or Seismic) = Seismic
load balance check = OK

2nd Story Walls (Front - Back Direction)
Hold downs and window straps

Story	Wall Mark	Wall L(ft)	Opening Width (ft)	Opening Height (ft)	Opening (max) to Edge (ft)	Plate to Opening (ft)	Effective Length (ft)	Trib. Width (ft)	Percent Sharing (%)	Effective Trib. Width	Story V(kips)	Sum V(kips)	Panel Shear (plf)	Height/Width Reduction (%) R = 2*L/H	Design Panel Shear (plf)	Wall Type	Roof DL Trib(ft)	Story DL(klf)	Sum DL(klf)	OTM (k-ft)	RM (k-ft)	Resultant HD(kips)	HD TYPE	HD/Strap to DF or HF?	HD location Edge/Interior?	Resultant HD	Force at Window (Kips)	Window Strap
2	1.0	18.75	5.50	2.50	3.00	2.00	13.25	10.00	1.00	10.00	2.12	2.12	160	1.00	160	SW6	2.00	0.14	0.14	19.0	21.8	-0.15	flr-flr	HF	Edge	No HD	0.78	CS16
2	2.1	7.00					7.00	25.00	0.47	11.67	2.47	2.47	353	1.00	353	SW3	2.00	0.14	0.14	24.7	3.0	3.33	flr-beam	HF	Edge	HDU5	0.00	No strap
2	2.2	8.00					8.00	25.00	0.53	13.33	2.82	2.82	353	1.00	353	SW3	2.00	0.14	0.14	39.5	4.0	4.74	flr-flr	HF	Edge	CMST14	0.00	No strap
2	3.0 (Assumed Existing)	22.00					22.00	14.00	1.00	14.00	2.96	2.96	135	1.00	135	SW6	4.00	0.17	0.17	29.6	36.6	-0.32	flr-flr	HF	Edge	No HD	0.00	No strap
	S =	55.75				Total OSB wall length = (feet)	50.25		S =	49.00	10.37	10.37	OK	Total OSB Capacity (kips)	10.37													

1st Story Walls (Front - Back Direction)

Shear panel capacity (Wind or Seismic) = Wind

"Adjusted" Story shear(kips) = 8.48
Story height (ft) = 10.00
Shear Panel height (ft) = 9.00
Total Diaphragm Width (ft) = 49.00

Accumulated Shear = 18.85
load balance check = OK

1st Story Walls (Front - Back Direction)
Hold downs and window straps

Story	Wall Mark	Wall L(ft)	Opening Width (ft)	Opening Height (ft)	Opening (max) to Edge (ft)	Plate to Opening (ft)	Effective Length (ft)	Trib. Width (ft)	Percent Sharing (%)	Effective Trib. Width	Story V(kips)	Sum V(kips)	Panel Shear (plf)	Height/Width Reduction (%) R = 2*L/H	Design Panel Shear (plf)	Wall Type	Floor DL Trib(ft)	Story DL(klf)	Walls/DL Stacks?	Sum DL(klf)	OTM (k-ft)	RM (k-ft)	Resultant HD(kips)	HD TYPE	HD/Strap to DF or HF?	HD location Edge/Interior?	Resultant HD	Force at Window (Kips)	Window Strap
1	1.0	39.00	6.00	2.00	5.00	2.00	33.00	12.00	1.00	12.00	2.08	4.19	127	1.00	127	SW6	2.00	0.13	YES	0.27	41.9	184.8	-3.86	flr-conc	HF	Edge	No HD	0.95	CS16
1	2.1	9.50					9.50	23.00	0.56	12.85	2.22	4.69	494	1.00	494	SW2	2.00	0.13	YES	0.27	46.9	11.0	4.00	flr-conc	HF	Edge	HDU5	0.00	No strap
1	2.2	7.50					7.50	23.00	0.44	10.15	1.76	4.58	610	1.00	610	2W4	2.00	0.13	YES	0.27	45.8	6.8	5.56	flr-conc	HF	Edge	HDU8	0.00	No strap
1	3.0	10.67					10.67	14.00	1.00	14.00	2.42	5.38	505	1.00	505	SW2	2.00	0.13	YES	0.30	53.8	15.4	3.78	flr-conc	HF	Edge	HDU5	0.00	No strap
	S =	66.67				Total OSB wall length = (feet)	60.67		S =	49.00	8.48	18.85	OK	Total OSB Capacity (kips)	8.48													

Project Number:	Plan Name:	Sheet Number:
S221118-2	Litchfield Residence	L6
Engineer:	Specifics:	Date:
HK	Shear walls	4/2/2023

Notes:

* All walls designed with Force-Transfer should meet a minimum height to width ratio of 2:1 at Pier (SDPWS 2018, Table 4.3.4)
* Maximum allowed height to width ratio 3.5:1 for walls w/o openings (increased shear design values per SDPWS 2018, Table 4.3.4)
* Shear panel height is height to underside or roof or floor framing.

RED = Update Formula as required - Important
BLUE = Review and update as required - Typical Input

2nd Story Walls (Side / Side Direction)

Stud Species: HF

"Adjusted" Story shear(kips) = 10.37
Story height (ft) = 9.08
Shear Panel height (ft) = 8.08
Total Diaphragm width (ft) = 45.00

100% story shear YES

Governing Force (F/B Direction) = Seismic
Dead load factor (F/B Direction) = 0.90 IBC 2018 Equation 16-22
Shear panel capacity (Wind or Seismic) = Seismic
load balance check = Warning-Wall loads do not match story shear

2nd Story Walls (Side / Side Direction)
Hold downs and window straps

Story	Wall Mark	Wall L(ft)	Opening Width (ft)	Opening Height (ft)	Opening (max) to Edge (ft)	Plate to Opening (ft)	Effective Length (ft)	Trib. Width (ft)	Percent Sharing (%)	Effective Trib. Width	Story V(kips)	Sum V(kips)	Panel Shear (plf)	Height/Width Reduction (%) R = 2*L/H	Design Panel Shear (plf)	Wall Type	Roof DL Trib(ft)	Story DL(klf)	Sum DL(klf)	OTM (k-ft)	RM (k-ft)	Resultant HD(kips)	HD TYPE	HD/Strap to DF or HF?	HD location Edge/Interior?	Resultant HD	Force at Window (Kips)	Window Strap
2	A (Assumed Existing)	22.00					22.00	13.00	1.00	13.00	2.99	2.99	136	1.00	136	SW6	2.00	0.13	0.13	27.2	27.7	-0.02	flr-flr	HF	Edge	No HD	0.00	No strap
2	B1	19.00	10.50	4.00	4.00	2.00	8.50	21.00	0.44	9.27	2.14	2.14	251	1.00	251	SW4	2.00	0.13	0.13	19.4	20.6	-0.07	flr-beam	HF	Edge	No HD	2.01	CS14
2	B2	10.75					10.75	21.00	0.56	11.73	2.70	2.70	251	1.00	251	SW4	2.00	0.13	0.13	24.5	6.6	1.75	flr-flr	HF	Edge	MST37	0.00	No strap
2	C	11.50					11.50	11.00	1.00	11.00	2.53	2.53	220	1.00	220	SW6	2.00	0.13	0.13	23.0	7.6	1.40	flr-beam	HF	Edge	MSTC48B3	0.00	No strap
2	D1	5.50					5.50	14.00	0.32	4.53	1.04	1.04	190	1.00	190	SW6	12.00	0.28	0.28	9.5	3.8	1.14	flr-beam	HF	Edge	MSTC48B3	0.00	No strap
3	D2 (Assumed Existing)	11.50					11.50	14.00	0.68	9.47	2.18	2.18	190	1.00	190	SW6	2.00	0.13	0.13	19.8	7.6	1.11	flr-flr	HF	Edge	MST37	0.00	No strap
	S =	80.25				Total OSB wall length = (feet)	52.75			S =	59.00	13.59	**13.59**	Warning-Wal	Total OSB Capacity (kips)	**10.37**													

1st Story Walls (Side / Side Direction)

Shear panel capacity (Wind or Seismic) = Wind

"Adjusted" Story shear(kips) = 7.83
Story height (ft) = 9.08
Shear Panel height (ft) = 8.08
Total Diaphragm width (ft) = 45.00

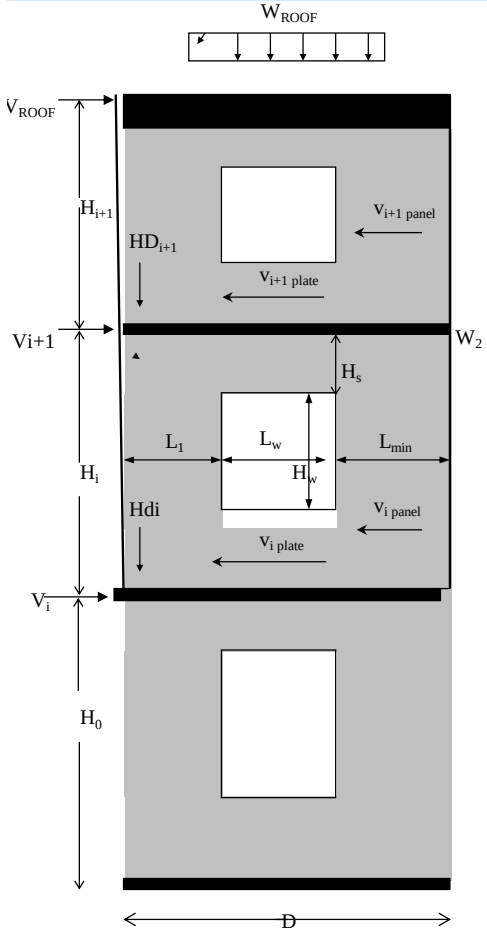
Accumulated Shear = 18.20
load balance check = Warning-Wall loads do not match story shear

1st Story Walls (Side / Side Direction)
Hold downs and window straps

Story	Wall Mark	Wall L(ft)	Opening Width (ft)	Opening Height (ft)	Opening (max) to Edge (ft)	Plate to Opening (ft)	Effective Length (ft)	Trib. Width (ft)	Percent Sharing (%)	Effective Trib. Width	Story V(kips)	Sum V(kips)	Panel Shear (plf)	Height/Width Reduction (%) R = 2*L/H	Design Panel Shear (plf)	Wall Type	Floor DL Trib(ft)	Story DL(klf)	Walls/DL Stacks?	Sum DL(klf)	OTM (k-ft)	RM (k-ft)	Resultant HD(kips)	HD TYPE	HD/Strap to DF or HF?	HD location Edge/Interior?	Resultant HD	Force at Window (Kips)	Window Strap
1	A	24.00					24.00	13.00	1.00	13.00	2.26	5.26	219	1.00	219	SW6	2.00	0.12	NO	0.12	47.7	30.6	0.73	flr-conc	HF	Edge	STHD14	0.00	No strap
1	B1	1.50					1.50	21.00	0.11	2.29	0.40	0.93	618		SIMPSON STRONG WALL														
1	B2	1.50					1.50	21.00	0.11	2.29	0.40	0.93	618		SIMPSON STRONG WALL														
2	B3	10.75					10.75	21.00	0.78	16.42	2.86	6.64	618	1.00	618	2W4	4.00	0.14	NO	0.14	60.3	7.4	1.70	flr-conc	DF-L	Edge	STHD14	0.00	No strap
1	C	18.50					18.50	11.00	1.00	11.00	1.92	4.45	240	1.00	240	SW4	4.00	0.14	NO	0.14	40.4	21.9	-2.44	flr-conc	DF-L	Edge	No HD	0.00	No strap
1	D	26.50	11.50	3.00	3.50	2.00	15.00	14.00	1.00	14.00	2.44	5.66	377	1.00	377	SW3	5.00	0.15	NO	0.15	51.4	48.7	-3.36	flr-conc	DF-L	Edge	No HD	2.31	CS14
	S =	82.75				Total OSB wall length = (feet)	27.00			S =	59.00	10.27	**23.86**	Warning-Wal	Total OSB Capacity (kips)	**7.83**													

Project	sheet number:
Litchfield Residence	**L7**
Subject	Date
SHEAR WALL EQUATION DIAGRAM	**4/2/2023**

SHEAR WALL WITH WINDOW BASED ON SHEAR TRANSFER:



Where:

V_i = Story Shear
W_i = Story Dead Load
HD_i = Story Holdown
M_{OTi} = Story Over Turning Moment
M_{Ri} = Story Resisting Moment

$M_{OT\ ROOF} = V_{ROOF} \times H_{1+1}$

$M_{OTi} = [(V_{i+1} + V_{ROOF}) \times H_i] + M_{OT\ RO}$

$M_{R\ ROOF} = 0.6 \times W_{ROOF} \times D^2/2$

$M_{Ri} = 0.6 \times (W_{i+1} + W_{ROOF}) \times D^2/2$

$HD_{i+1} = (M_{OT\ ROOF} - M_{R\ ROOF})/(D - 6")$

$HD_i = (M_{OTi} - M_{R1i})/(D - 6")$

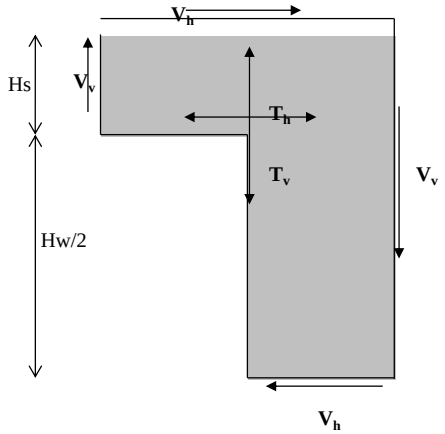
$V_{i+1\ panel} = V_{ROOF} / (L_1 + L_{max})$

$V_{i\ panel} = (V_{ROOF} + V_{i+1}) / (L_1 + L_{max})$

$V_{i+1\ plate} = V_{ROOF} / D$

$V_{i\ plate} = (V_{ROOF} + V_{i+1}) / D$

FORCE TRANSFER AROUND WINDOW CALCULATION (CANTILEVER PIER METHOD)



$V_h = v_{i\ panel} \times L_{max}$

$V_v = HD_i$

$T_h = V_h (H_w / 2 + H_s) / H_s$

T_v = Is resisted by the continuous stud adjacent to the window.

LONGITUDE
ONE TWENTY°
ENGINEERING & DESIGN

FOUNDATION CALCULATIONS

FOOTING REFERENCE PER PLAN

(425) 636 3313
L120Engineering.com

Project Title:
Engineer:
Project ID:
Project Descr:

Wall Footing

DESCRIPTION: 1'-4" (16") Footing and Stem-wall (non retaining) - Max Loading (1500psf)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : ASCE 7-16

General Information

Material Properties

f'c : Concrete 28 day strength	=	2.50 ksi
fy : Rebar Yield	=	40.0 ksi
Ec : Concrete Elastic Modulus	=	3,122.0 ksi
Concrete Density	=	145.0 pcf
φ Values Flexure	=	0.90
Shear	=	0.750

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
AutoCalc Footing Weight as DL :		Yes

Soil Design Values

Allowable Soil Bearing	=	1.50 ksf
Increase Bearing By Footing Weight	=	No
Soil Passive Resistance (for Sliding)	=	300.0 pcf
Soil/Concrete Friction Coeff.	=	0.30

Increases based on footing Depth

Reference Depth below Surface	=	ft
Allow. Pressure Increase per foot of depth	=	ksf
when base footing is below	=	ft

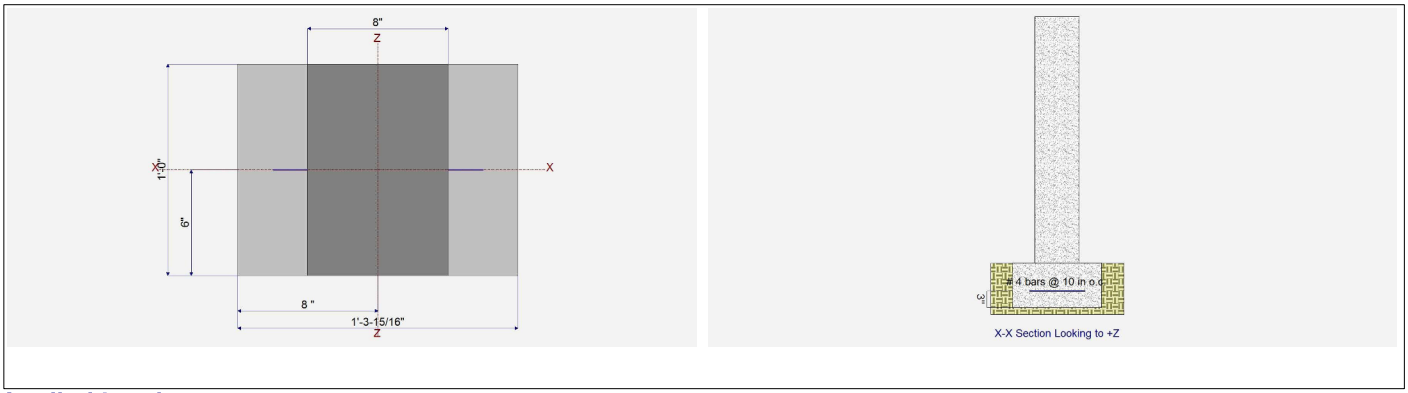
Increases based on footing Width

Allow. Pressure Increase per foot of width	=	ksf
when footing is wider than	=	ft
Adjusted Allowable Bearing Pressure	=	1.50 ksf

Dimensions

Footing Width	=	1.330 ft	Footing Thickness	=	8.0 in	Bars along X-X Axis	
Wall Thickness	=	8.0 in	Rebar Centerline to Edge of Concrete...			Bar spacing	= 10.00
Wall center offset from center of footing	=	0 in	at Bottom of footing	=	3.0 in	Reinforcing Bar Size	= # 4

Reinforcing



Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	1.0		0.750	0.40				k
OB : Overburden	=								ksf
V-x	=								k
M-zz	=								k-ft

Vx applied = in above top of footing

MAX POSSIBLE LOADING ONTO NEW OR EXISTING FOUNDATION =
ULTIMATE = 100 LB (D) + 320 LB (L)
FACTORED = 1.2D + 1.6L = 632 PLF

ALLOWABLE = 1800 PLF FACTORED > 632 PLF

THEREFORE FOOTING DESIGN OK FOR MAX POSSIBLE LOADING

Project Title:
Engineer:
Project ID:
Project Descr:

Wall Footing

DESCRIPTION: 1'-4" (16") Footing and Stem-wall (non retaining) - Max Loading (1500psf)

DESIGN SUMMARY

Design OK

	Factor of Safety	Item	Applied	Capacity	Governing Load Combination
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift

	Utilization Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9980	Soil Bearing	1.497 ksf	1.50 ksf	+D+0.750L+0.750S
PASS	0.03287	Z Flexure (+X)	0.1139 k-ft	3.464 k-ft	+1.20D+1.60L+0.50S
PASS	0.02607	Z Flexure (-X)	0.09031 k-ft	3.464 k-ft	+1.20D+L+0.20S
PASS	n/a	1-way Shear (+X)	0.0 psi	75.0 psi	n/a
PASS	0.0	1-way Shear (-X)	0.0 psi	0.0 psi	n/a

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc	Actual Soil Bearing Stress -X	Actual Soil Bearing Stress +X	Actual / Allowable Ratio
, D Only	1.50 ksf	0.0 in	0.8485 ksf	0.8485 ksf	0.566
, +D+L	1.50 ksf	0.0 in	1.412 ksf	1.412 ksf	0.942
, +D+S	1.50 ksf	0.0 in	1.149 ksf	1.149 ksf	0.766
, +D+0.750L	1.50 ksf	0.0 in	1.271 ksf	1.271 ksf	0.848
, +D+0.750L+0.750S	1.50 ksf	0.0 in	1.497 ksf	1.497 ksf	0.998
, +0.60D	1.50 ksf	0.0 in	0.5091 ksf	0.5091 ksf	0.339

Overturning Stability

Units : k-ft

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Which Side ?	Tension @ Bot. or Top ?	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
, +1.40D	0.06532	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.40D	0.06532	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+1.60L	0.1056	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+1.60L	0.1056	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+1.60L+0.50S	0.1139	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+1.60L+0.50S	0.1139	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+L	0.087	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+L	0.087	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D	0.05599	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D	0.05599	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+L+1.60S	0.1135	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+L+1.60S	0.1135	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+1.60S	0.08245	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+1.60S	0.08245	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+L+0.50S	0.09527	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+L+0.50S	0.09527	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +0.90D	0.04199	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +0.90D	0.04199	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+L+0.20S	0.09031	-X	Bottom	0.1728	Min Temp %	0.24	3.464	OK
, +1.20D+L+0.20S	0.09031	+X	Bottom	0.1728	Min Temp %	0.24	3.464	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

LIC# : KW-06011993, Build:20.22.1.5 L120 Engineering and Design

DESCRIPTION: 16" (non retaining) stemwall footing - max point load (1500psf)

Code References

Calculations per ACI 318-14, IBC 2018, CBC 2019, ASCE 7-16
Load Combinations Used : IBC 2018

General Information

Material Properties

f'c : Concrete 28 day strength	=	2.5 ksi
fy : Rebar Yield	=	60.0 ksi
Ec : Concrete Elastic Modulus	=	3,122.0 ksi
Concrete Density	=	145.0 pcf
φ Values Flexure	=	0.90
Shear	=	0.750

Analysis Settings

Min Steel % Bending Reinf.	=	
Min Allow % Temp Reinf.	=	0.00180
Min. Overturning Safety Factor	=	1.0 : 1
Min. Sliding Safety Factor	=	1.0 : 1
Add Ftg Wt for Soil Pressure	:	Yes
Use ftg wt for stability, moments & shears	:	Yes
Add Pedestal Wt for Soil Pressure	:	No
Use Pedestal wt for stability, mom & shear	:	No

Soil Design Values

Allowable Soil Bearing	=	1.50 ksf
Soil Density	=	110.0 pcf
Increase Bearing By Footing Weight	=	No
Soil Passive Resistance (for Sliding)	=	250.0 pcf
Soil/Concrete Friction Coeff.	=	0.30

Increases based on footing Depth

Footing base depth below soil surface	=	1.0 ft
Allow press. increase per foot of depth	=	ksf
when footing base is below	=	ft

Increases based on footing plan dimension

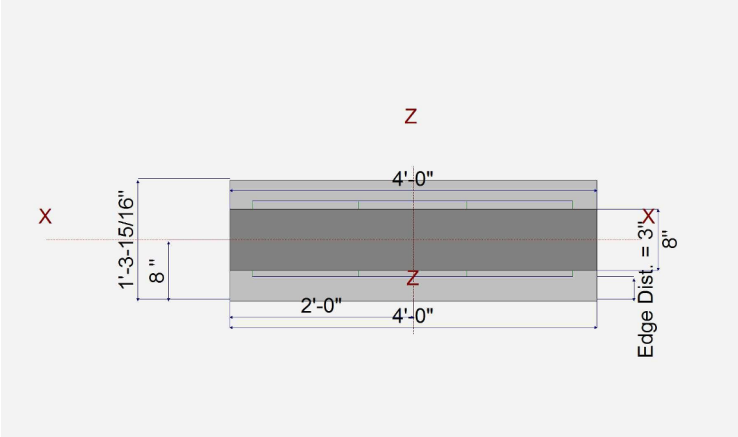
Allowable pressure increase per foot of depth	=	ksf
when max. length or width is greater than	=	ft

Dimensions

Width parallel to X-X Axis	=	4.0 ft
Length parallel to Z-Z Axis	=	1.330 ft
Footing Thickness	=	8.0 in

Pedestal dimensions...

px : parallel to X-X Axis	=	48.0 in
pz : parallel to Z-Z Axis	=	8.0 in
Height	=	18.0 in
Rebar Centerline to Edge of Concrete... at Bottom of footing	=	3.0 in



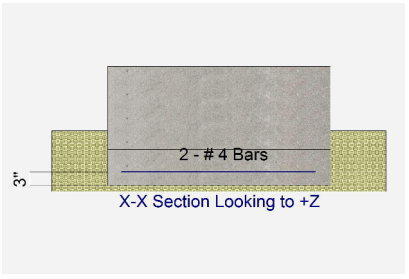
Reinforcing

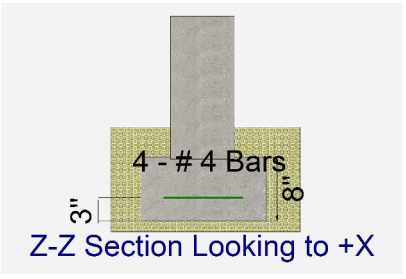
Bars parallel to X-X Axis			
Number of Bars	=		2.0
Reinforcing Bar Size	=	#	4
Bars parallel to Z-Z Axis			
Number of Bars	=		4.0
Reinforcing Bar Size	=	#	4

Bandwidth Distribution Check (ACI 15.4.4.2)

Direction Requiring Closer Separation

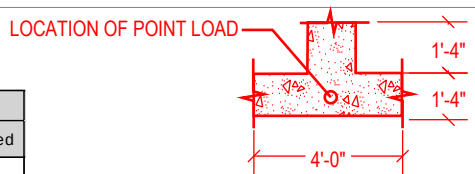
	Bars along Z-Z Axis
# Bars required within zone	49.9 %
# Bars required on each side of zone	50.1 %





Applied Loads

		D	Lr	L	S	W	E	H	
P : Column Load	=	3.0		4.30					k
OB : Overburden	=								ksf
M-xx	=								k-ft
M-zz	=								k-ft
V-x	=								k
V-z	=								k



Loads to Supports (lbs)			
Dead	Floor Live	Snow	Factored
4084	2391	2273	7583
4142	2654	2372	7912

GOVERNING DESIGN PARAMETER IS SOIL BEARING CAPACITY = 1500 PSF. THEREFORE W/FOOTPRINT AREA OF 7 SQ. FT. TOTAL SOIL BEARING CAPACITY = 10.5K > FACTORED LOAD APPLIED. THEREFORE DESIGN OK!

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

LIC# : KW-06011993, Build:20.22.1.5 L120 Engineering and Design

DESCRIPTION: 16" (non retaining) stemwall footing - max point load (1500psf)

DESIGN SUMMARY

Design OK

	Min. Ratio	Item	Applied	Capacity	Governing Load Combination
PASS	0.9913	Soil Bearing	1.487 ksf	1.50 ksf	+D+L about Z-Z axis
PASS	n/a	Overturning - X-X	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Overturning - Z-Z	0.0 k-ft	0.0 k-ft	No Overturning
PASS	n/a	Sliding - X-X	0.0 k	0.0 k	No Sliding
PASS	n/a	Sliding - Z-Z	0.0 k	0.0 k	No Sliding
PASS	n/a	Uplift	0.0 k	0.0 k	No Uplift
PASS	0.0	Z Flexure (+X)	0.0 k-ft/ft	0.0 k-ft/ft	No Moment
PASS	0.0	Z Flexure (-X)	0.0 k-ft/ft	0.0 k-ft/ft	No Moment
PASS	0.02530	X Flexure (+Z)	0.1071 k-ft/ft	4.235 k-ft/ft	+1.20D+1.60L
PASS	0.02530	X Flexure (-Z)	0.1071 k-ft/ft	4.235 k-ft/ft	+1.20D+1.60L
PASS	n/a	1-way Shear (+X)	0.0 psi	67.082 psi	n/a
PASS	n/a	1-way Shear (-X)	0.0 psi	67.082 psi	n/a
PASS	n/a	1-way Shear (+Z)	0.0 psi	67.082 psi	n/a
PASS	n/a	1-way Shear (-Z)	0.0 psi	67.082 psi	n/a
PASS	n/a	2-way Punching	0.0 psi	67.082 psi	n/a

Detailed Results

Soil Bearing

Rotation Axis & Load Combination...	Gross Allowable	Xecc (in)	Zecc (in)	Actual Soil Bearing Stress @ Location Bottom, -Z	Top, +Z	Left, -X	Right, +X	Actual / Allow Ratio
X-X, D Only	1.50	n/a	0.0	0.6789	0.6789	n/a	n/a	0.453
X-X, +D+L	1.50	n/a	0.0	1.487	1.487	n/a	n/a	0.991
X-X, +D+0.750L	1.50	n/a	0.0	1.285	1.285	n/a	n/a	0.857
X-X, +0.60D	1.50	n/a	0.0	0.4073	0.4073	n/a	n/a	0.272
Z-Z, D Only	1.50	0.0	n/a	n/a	n/a	0.6789	0.6789	0.453
Z-Z, +D+L	1.50	0.0	n/a	n/a	n/a	1.487	1.487	0.991
Z-Z, +D+0.750L	1.50	0.0	n/a	n/a	n/a	1.285	1.285	0.857
Z-Z, +0.60D	1.50	0.0	n/a	n/a	n/a	0.4073	0.4073	0.272

Overturning Stability

Rotation Axis & Load Combination...	Overturning Moment	Resisting Moment	Stability Ratio	Status
Footing Has NO Overturning				

Sliding Stability

All units k

Force Application Axis Load Combination...	Sliding Force	Resisting Force	Stability Ratio	Status
Footing Has NO Sliding				

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
X-X, +1.40D	0.04201	+Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +1.40D	0.04201	-Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +1.20D+1.60L	0.1071	+Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +1.20D+1.60L	0.1071	-Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +1.20D+0.50L	0.05823	+Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +1.20D+0.50L	0.05823	-Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +1.20D	0.03601	+Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +1.20D	0.03601	-Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +0.90D	0.0270	+Z	Bottom	0.1728	AsMin	0.20	4.235	OK
X-X, +0.90D	0.0270	-Z	Bottom	0.1728	AsMin	0.20	4.235	OK
Z-Z, +1.40D	0.0	-X	Top	0.1728	AsMin	0.3008	6.168	OK
Z-Z, +1.40D	0.0	+X	Top	0.1728	AsMin	0.3008	6.168	OK
Z-Z, +1.20D+1.60L	0.0	-X	Top	0.1728	AsMin	0.3008	6.168	OK
Z-Z, +1.20D+1.60L	0.0	+X	Top	0.1728	AsMin	0.3008	6.168	OK
Z-Z, +1.20D+0.50L	0.0	-X	Top	0.1728	AsMin	0.3008	6.168	OK
Z-Z, +1.20D+0.50L	0.0	+X	Top	0.1728	AsMin	0.3008	6.168	OK
Z-Z, +1.20D	0.0	-X	Top	0.1728	AsMin	0.3008	6.168	OK

Project Title:
Engineer:
Project ID:
Project Descr:

General Footing

DESCRIPTION: 16" (non retaining) stemwall footing - max point load (1500psf)

Footing Flexure

Flexure Axis & Load Combination	Mu k-ft	Side	Tension Surface	As Req'd in^2	Gvrn. As in^2	Actual As in^2	Phi*Mn k-ft	Status
Z-Z, +1.20D	0.0	+X	Top	0.1728	AsMin	0.3008	6.168	OK
Z-Z, +0.90D	0.0	-X	Top	0.1728	AsMin	0.3008	6.168	OK
Z-Z, +0.90D	0.0	+X	Top	0.1728	AsMin	0.3008	6.168	OK

One Way Shear

Load Combination...	Vu @ -X	Vu @ +X	Vu @ -Z	Vu @ +Z	Vu:Max	Phi Vn	Vu / Phi*Vn	Status
+1.40D	0.00 psi	0.00 psi	0.00 psi	0.00 psi	0.00 psi	67.08 psi	0.00	OK
+1.20D+1.60L	0.00 psi	0.00 psi	0.00 psi	0.00 psi	0.00 psi	67.08 psi	0.00	OK
+1.20D+0.50L	0.00 psi	0.00 psi	0.00 psi	0.00 psi	0.00 psi	67.08 psi	0.00	OK
+1.20D	0.00 psi	0.00 psi	0.00 psi	0.00 psi	0.00 psi	67.08 psi	0.00	OK
+0.90D	0.00 psi	0.00 psi	0.00 psi	0.00 psi	0.00 psi	67.08 psi	0.00	OK

Two-Way "Punching" Shear

All units k

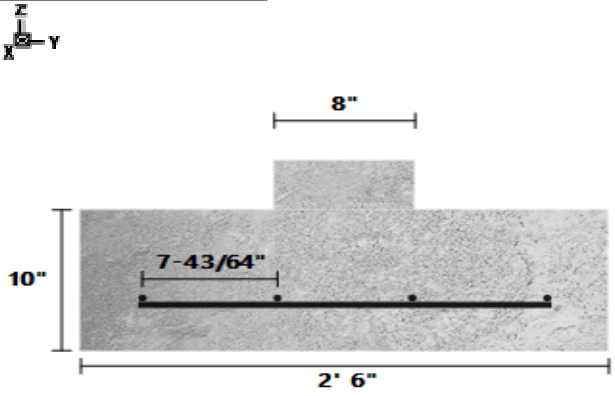
Load Combination...	Vu	Phi*Vn	Vu / Phi*Vn	Status
+1.40D	0.00 psi	89.44 psi	0	OK
+1.20D+1.60L	0.00 psi	89.44 psi	0	OK
+1.20D+0.50L	0.00 psi	89.44 psi	0	OK
+1.20D	0.00 psi	89.44 psi	0	OK
+0.90D	0.00 psi	89.44 psi	0	OK

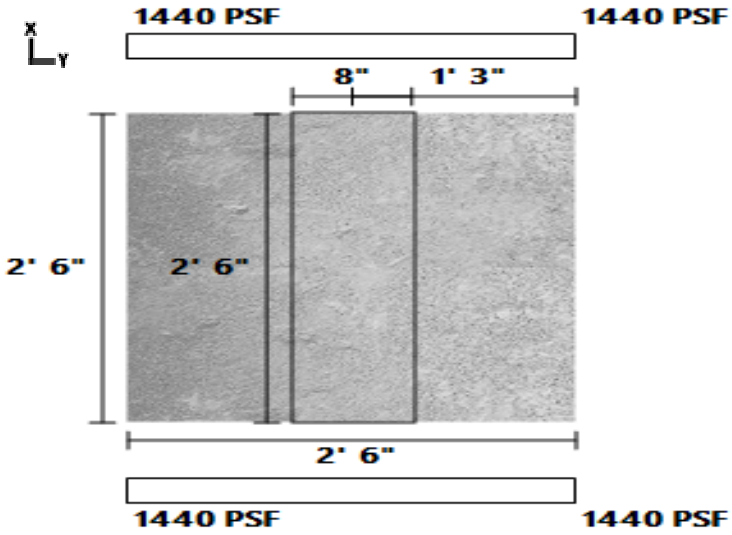
PASS

DATE:	2/11/2021	COMPANY:	L120 Engineering & Design, LLC
VITRUVIUS BUILD:	StruCalc	DESIGNED BY:	Mans Thurfjell
CUSTOMER:		REVIEWED BY:	Mans Thurfjell
PROJ. ADDRESS:	--	PROJECT NAME:	Foundation 1500psf
	--		
LEVEL:	Roof	LOADING:	
MEMBER NAME:	30x30x10	CODE:	2018 International Building Code
MEMBER TYPE:	ISOLATED FOOTING	ACI:	ACI 318-14
MATERIAL:	Concrete		

2.5 (ft) X 2.5 (ft) X 10 (in)	Soil Depth TOF: 0 (ft)	(4) #4 Long, (4) #4 Short

30x30x10 DIAGRAMS





MATERIAL PROPERTIES

FOOTING

fc' (psi)	Ec (psi)	Density (lbf/ft³)	Width (ft)	Length (ft)	Depth (in)	Volume (ft³)
2500	2880952	145	2.5	2.5	10	5.21

CALCULATION VARIABLES

Bo (in)	Φ-X	Φ-Y
0	0	0

COLUMN

Width (in)	Length (in)	Material	Offset (in)
8	30	Concrete	0

SOIL

Bearing Strength (lbf/ft²)	Density (lbf/ft³)	Cohesion	Friction Angle	Depth (ft)	Rankine Coefficient (Kp)
1500	140	0	30	0	3

REBAR

Bar Size #	# Bars Long	# Bars Short	fy (psi)	Es (psi)
4	4	4	40000	2.9E+07

PASS-FAIL

	PASS/FAIL	MAGNITUDE	STRENGTH	LOAD COMBO
Soil Bearing Pressure (lbf/ft²)	PASS (4.0%)	1440.0	1500.0	D+L
One-Way Shear Y (lbf)	PASS (87.1%)	1890.0	14625.0	1.2D+1.6L+0.5Lr
Moment Y (lbf-ft)	PASS (47.1%)	2117.5	4000.0	1.2D+1.6L+0.5Lr
Crushing (psi)	PASS (96.2%)	52.5	1381.3	1.2D+1.6L+0.5Lr

LOAD LIST

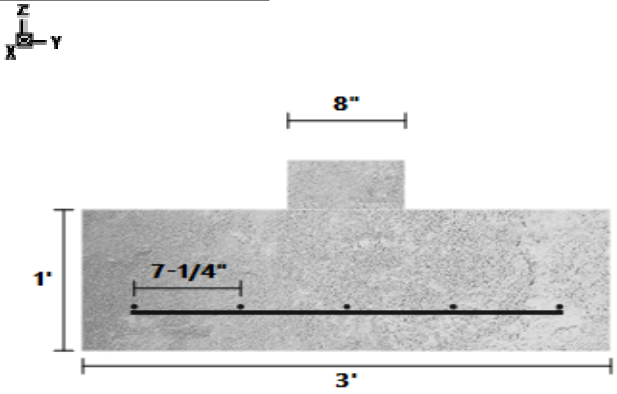
Type	Left Magnitude	Right Magnitude	Load Start (ft)	Load End (ft)	Load Type	Direction
Point (lbf)	4500	-	0	-	Live	Z
Point (lbf)	4500	-	0	-	Dead	Z

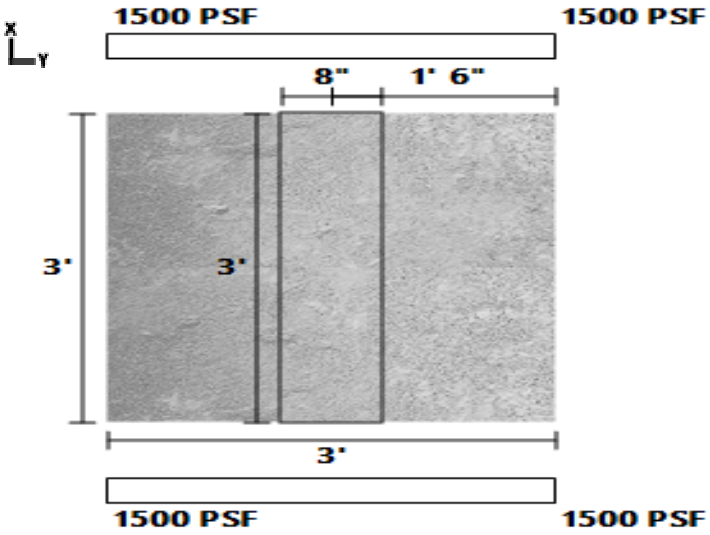
PASS

DATE:	2/11/2021	COMPANY:	L120 Engineering & Design, LLC
VITRUVIUS BUILD:	StruCalc	DESIGNED BY:	Mans Thurfjell
CUSTOMER:		REVIEWED BY:	Mans Thurfjell
PROJ. ADDRESS:	--	PROJECT NAME:	Foundation 1500psf
	--		
LEVEL:	Roof	LOADING:	
MEMBER NAME:	36x36x12	CODE:	2018 International Building Code
MEMBER TYPE:	ISOLATED FOOTING	ACI:	ACI 318-14
MATERIAL:	Concrete		

3 (ft) X 3 (ft) X 12 (in)	Soil Depth TOF: 0 (ft)	(5) #4 Long, (5) #4 Short

36x36x12 DIAGRAMS





MATERIAL PROPERTIES

FOOTING

fc' (psi)	Ec (psi)	Density (lbf/ft³)	Width (ft)	Length (ft)	Depth (in)	Volume (ft³)
2500	2880952	145	3	3	12	9

CALCULATION VARIABLES

Bo (in)	Φ-X	Φ-Y
0	0	0

COLUMN

Width (in)	Length (in)	Material	Offset (in)
8	36	Concrete	0

SOIL

Bearing Strength (lbf/ft²)	Density (lbf/ft³)	Cohesion	Friction Angle	Depth (ft)	Rankine Coefficient (Kp)
1500	140	0	30	0	3

REBAR

Bar Size #	# Bars Long	# Bars Short	fy (psi)	Es (psi)
4	5	5	40000	2.9E+07

PASS-FAIL

	PASS/FAIL	MAGNITUDE	STRENGTH	LOAD COMBO
Soil Bearing Pressure (lbf/ft²)	PASS (0.0%)	1500.0	1500.0	D+L
One-Way Shear Y (lbf)	PASS (87.4%)	2902.8	22950.0	1.2D+1.6L+0.5Lr
Moment Y (lbf-ft)	PASS (42.5%)	4310.2	7500.0	1.2D+1.6L+0.5Lr
Crushing (psi)	PASS (95.2%)	66.0	1381.3	1.2D+1.6L+0.5Lr

LOAD LIST

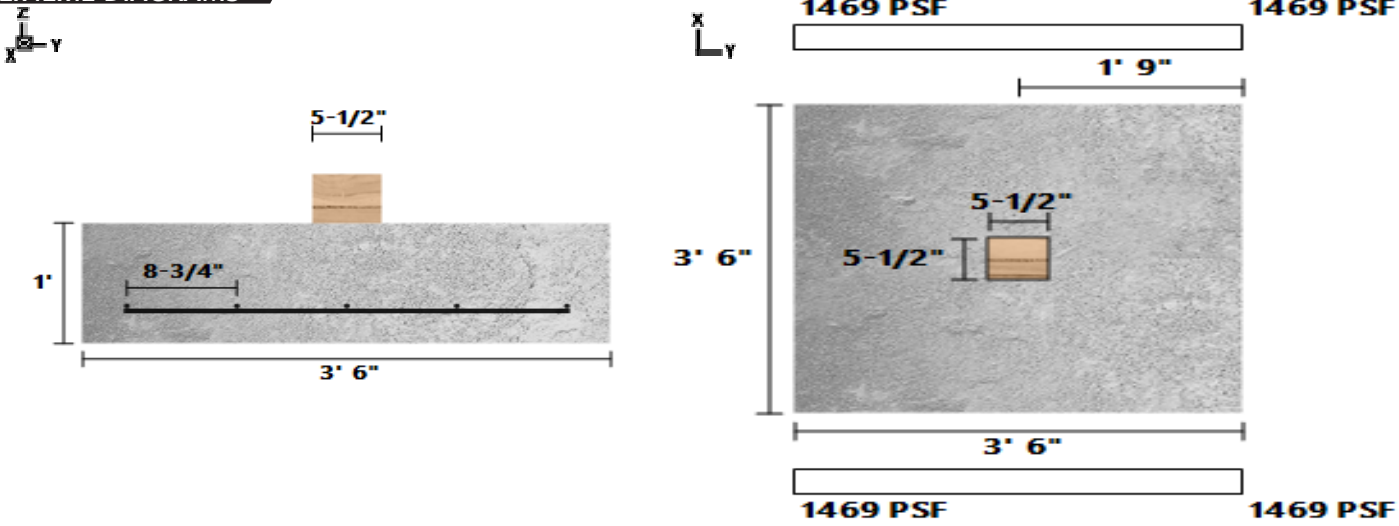
Type	Left Magnitude	Right Magnitude	Load Start (ft)	Load End (ft)	Load Type	Direction
Point (lbf)	7000	-	0	-	Live	Z
Point (lbf)	6500	-	0	-	Dead	Z

PASS

DATE:	10/8/2021	COMPANY:	L120 Engineering & Design, LLC
VITRUVIUS BUILD:	StruCalc	DESIGNED BY:	Mans Thurfjell
CUSTOMER:		REVIEWED BY:	Mans Thurfjell
PROJ. ADDRESS:	--	PROJECT NAME:	Foundation 1500psf
	--		
LEVEL:	Roof	LOADING:	
MEMBER NAME:	42x42x12	CODE:	2018 International Building Code
MEMBER TYPE:	ISOLATED FOOTING	ACI:	ACI 318-14
MATERIAL:	Concrete		

3.5 (ft) X 3.5 (ft) X 12 (in)	Soil Depth TOF: 0 (ft)	(5) #4 Long, (5) #4 Short

42x42x12 DIAGRAMS



MATERIAL PROPERTIES

FOOTING

fc' (psi)	Ec (psi)	Density (lbf/ft³)	Width (ft)	Length (ft)	Depth (in)	Volume (ft³)
2500	2880952	145	3.5	3.5	12	12.25

CALCULATION VARIABLES

Bo (in)	Φ-X	Φ-Y
56	0	0

COLUMN

Width (in)	Length (in)	Material	Offset (in)
5.5	5.5	Wood	0

SOIL

Bearing Strength (lbf/ft²)	Density (lbf/ft³)	Cohesion	Friction Angle	Depth (ft)	Rankine Coefficient (Kp)
1500	140	0	30	0	3

REBAR

Bar Size #	# Bars Long	# Bars Short	fy (psi)	Es (psi)
4	5	5	40000	2.9E+07

PASS-FAIL

	PASS/FAIL	MAGNITUDE	STRENGTH	LOAD COMBO
Soil Bearing Pressure (lbf/ft²)	PASS (2.0%)	1469.4	1500.0	D+L
Two-Way Shear (Punching) (lbf)	PASS (59.7%)	28800.0	71400.0	1.2D+1.6L+0.5Lr
One-Way Shear X (lbf)	PASS (75.0%)	6685.7	26775.0	1.2D+1.6L+0.5Lr
Moment X (lbf-ft)	PASS (61.7%)	9516.1	24827.7	1.2D+1.6L+0.5Lr
One-Way Shear Y (lbf)	PASS (75.0%)	6685.7	26775.0	1.2D+1.6L+0.5Lr
Moment Y (lbf-ft)	PASS (61.7%)	9516.1	24827.7	1.2D+1.6L+0.5Lr
Crushing (psi)	PASS (31.1%)	952.1	1381.3	1.2D+1.6L+0.5Lr

LOAD LIST

Type	Left Magnitude	Right Magnitude	Load Start (ft)	Load End (ft)	Load Type	Direction
Point (lbf)	9000	-	0	-	Live	Z
Point (lbf)	9000	-	0	-	Live	Z